



Wisdome – Technisches Museum Stockholm

27. Internationales Holzbau-Forum 2023 - Innsbruck

Stefan Rick – vertreten durch Philipp Schmon

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Der Wettbewerb



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Der Wettbewerb



Tekniska museet
Wissenschaftsmuseum
Bauherr: **Tekniska Museet**, Stockholm, Schweden

Architekt: **Elding Oscarson Architects**, Stockholm, Schweden

Design Tragstruktur & Bauingenieur: **Florian Kosche AS**, Oslo, Norwegen

Materialsponsor & Hauptpartner: **Stora Enso**, Schweden

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Das Siegerprojekt



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Das Team



Hermann Blumer



David Riggensch



Martin Antemann



Fabian Scheurer



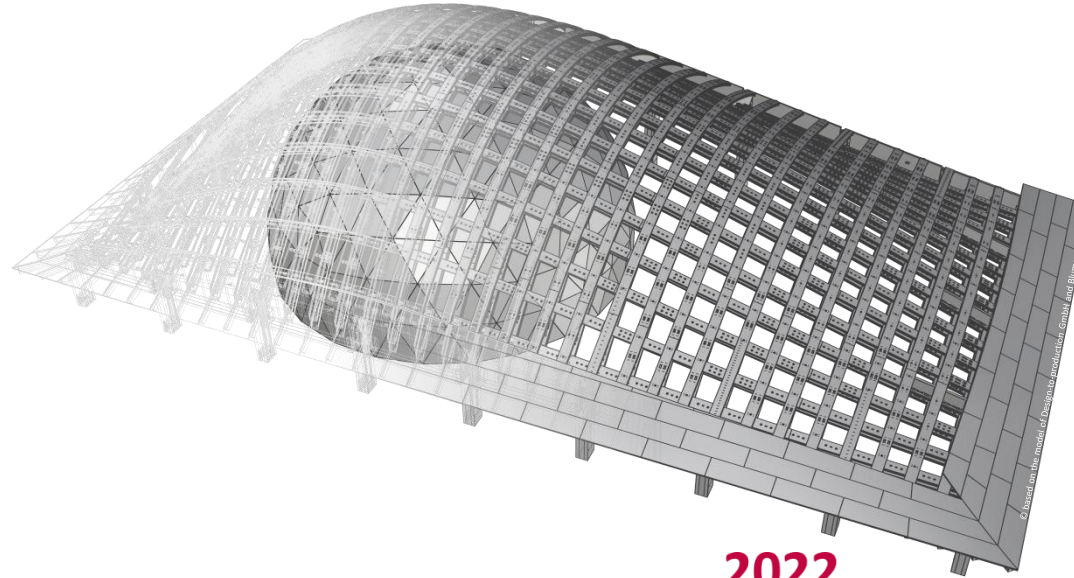
Stefan Rick



Evy Slabbinck

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Das Projekt

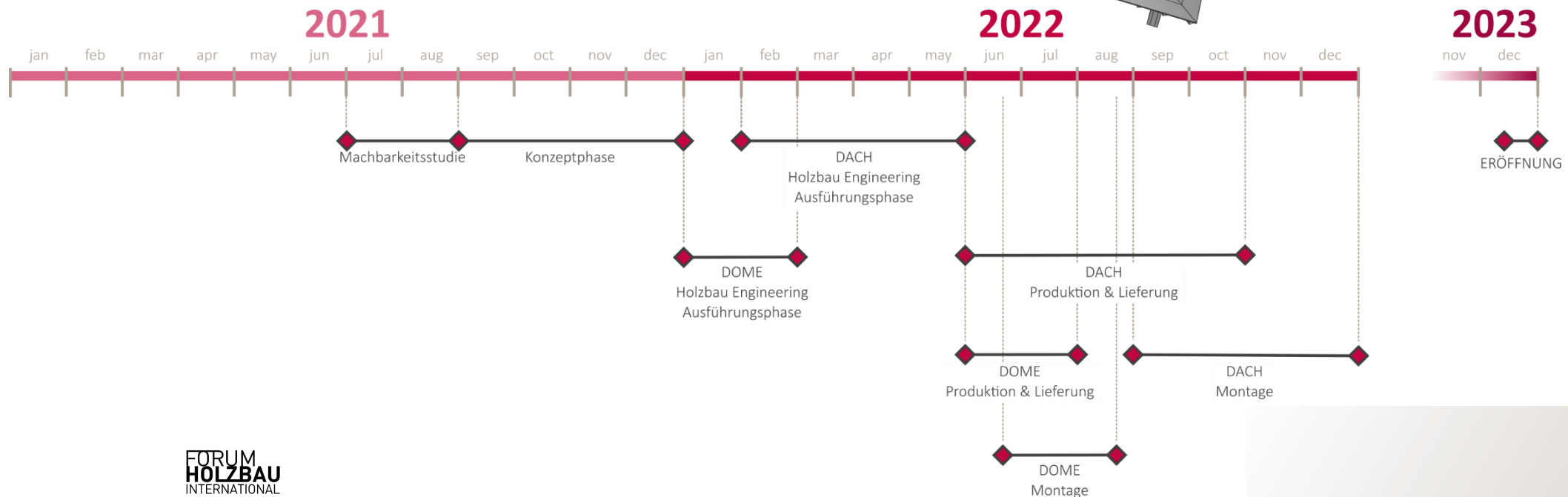


création
holz

Blumer
Lehmann

design
to
production

sib kempter
fitze



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Die Randbedingung...

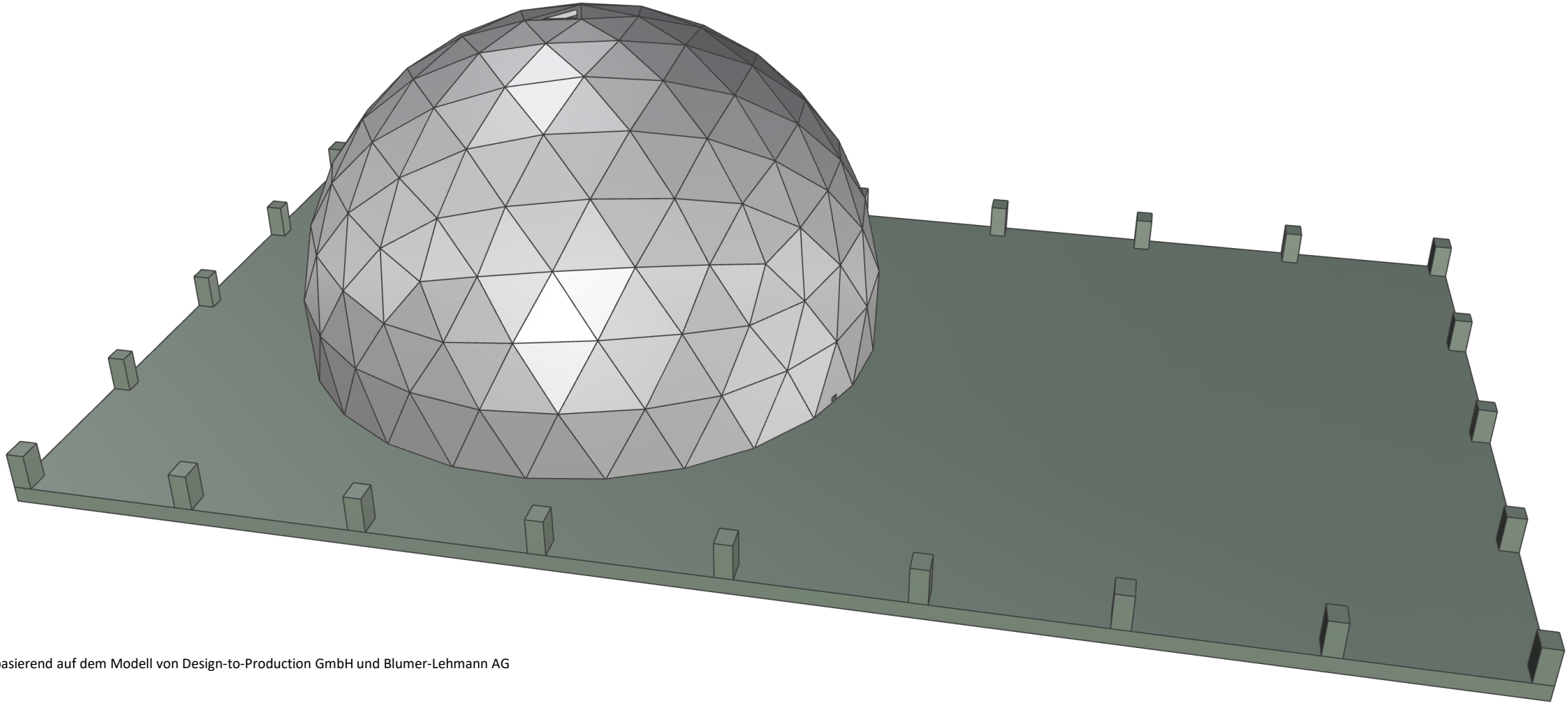


—————> Brettsperrholz (BSP)

—————> Furnierschichtholz (FSH)

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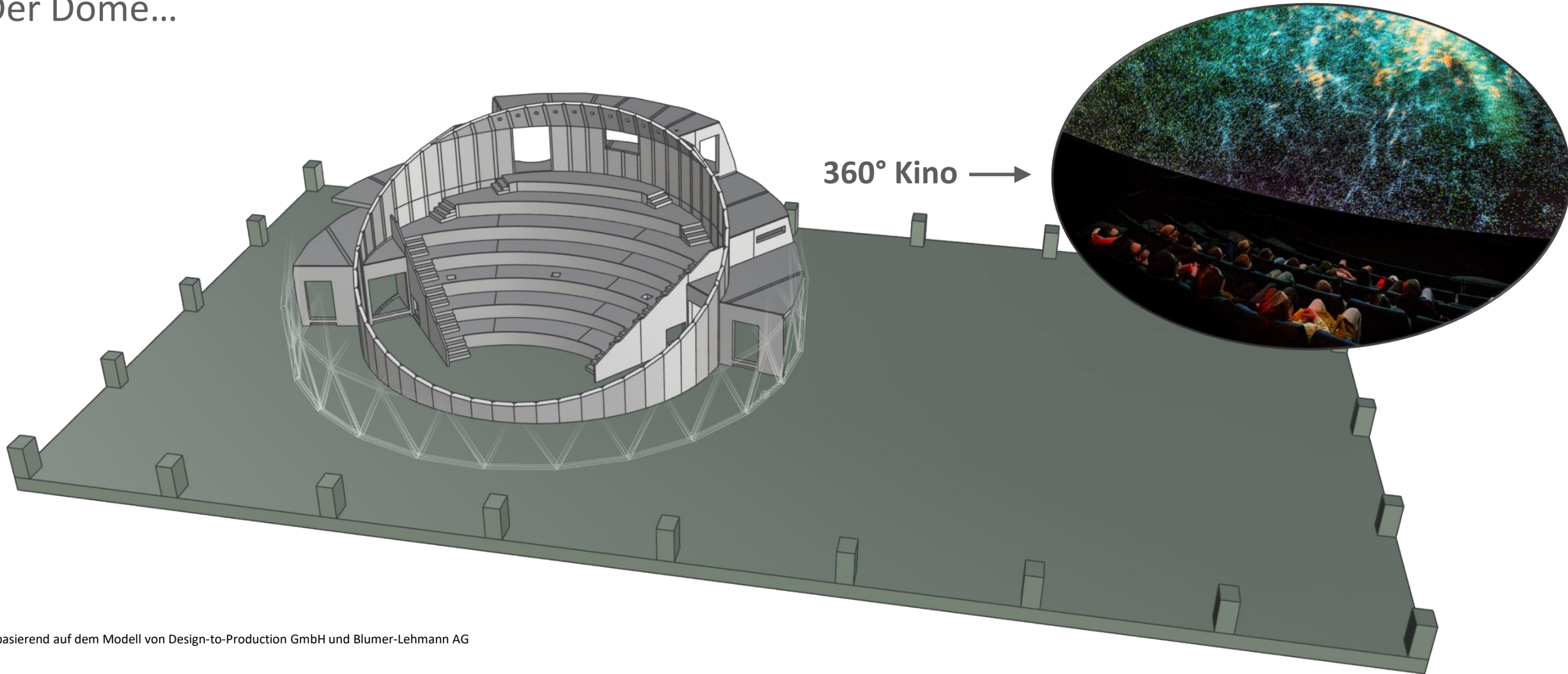
Der Dome...



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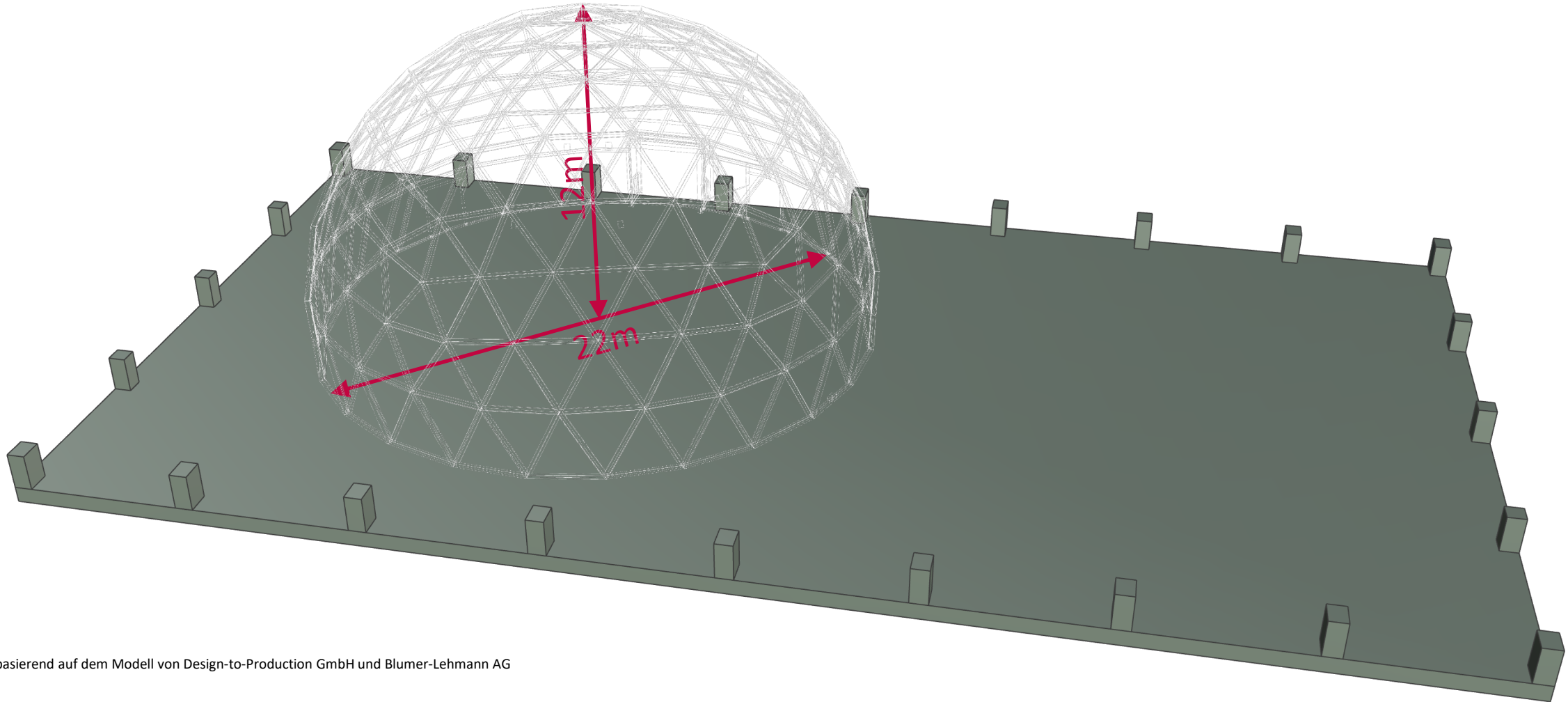
Der Dome...



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Der Dome...



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Das Dach...



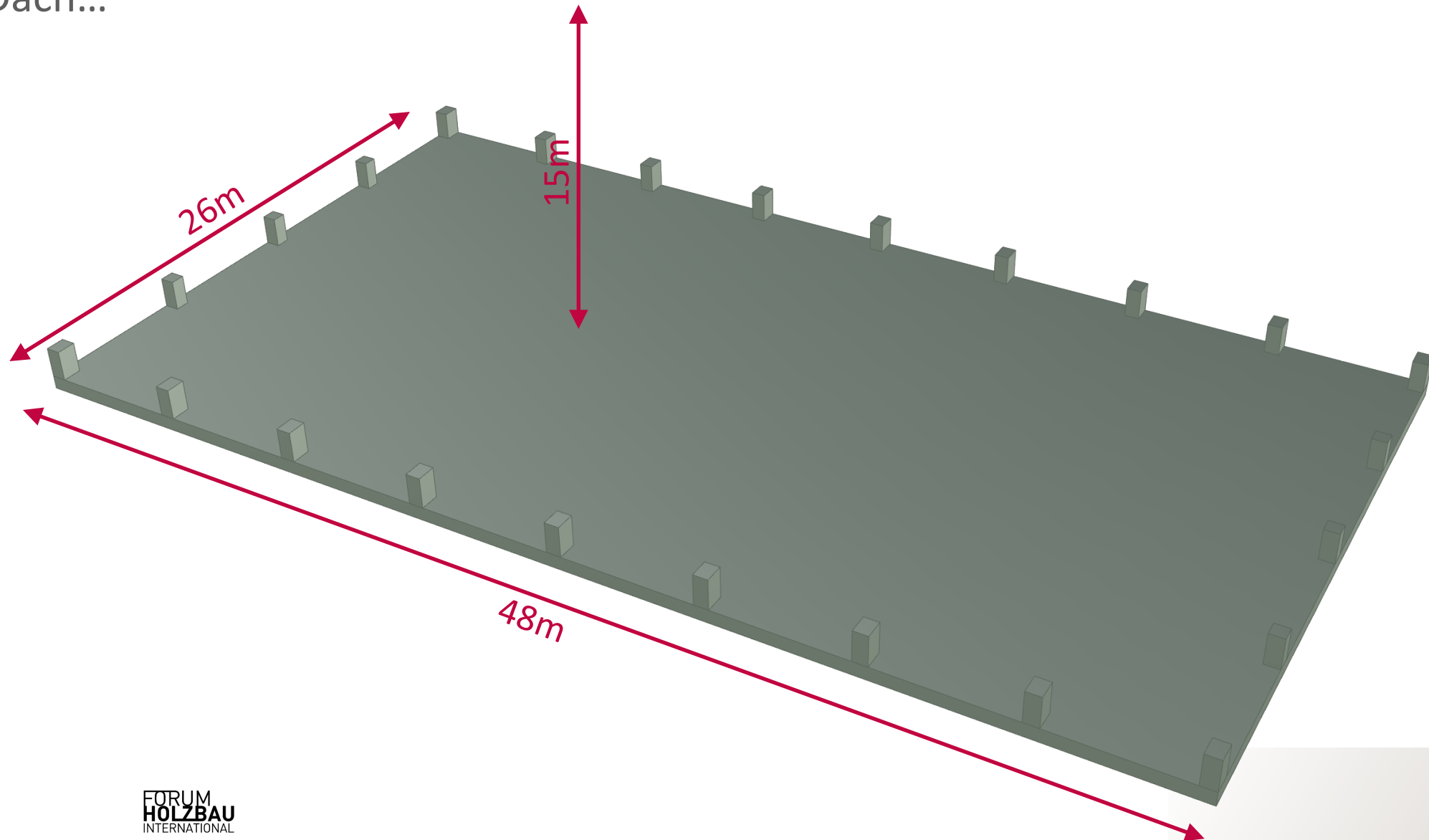
+



→ **Wie geht das?**

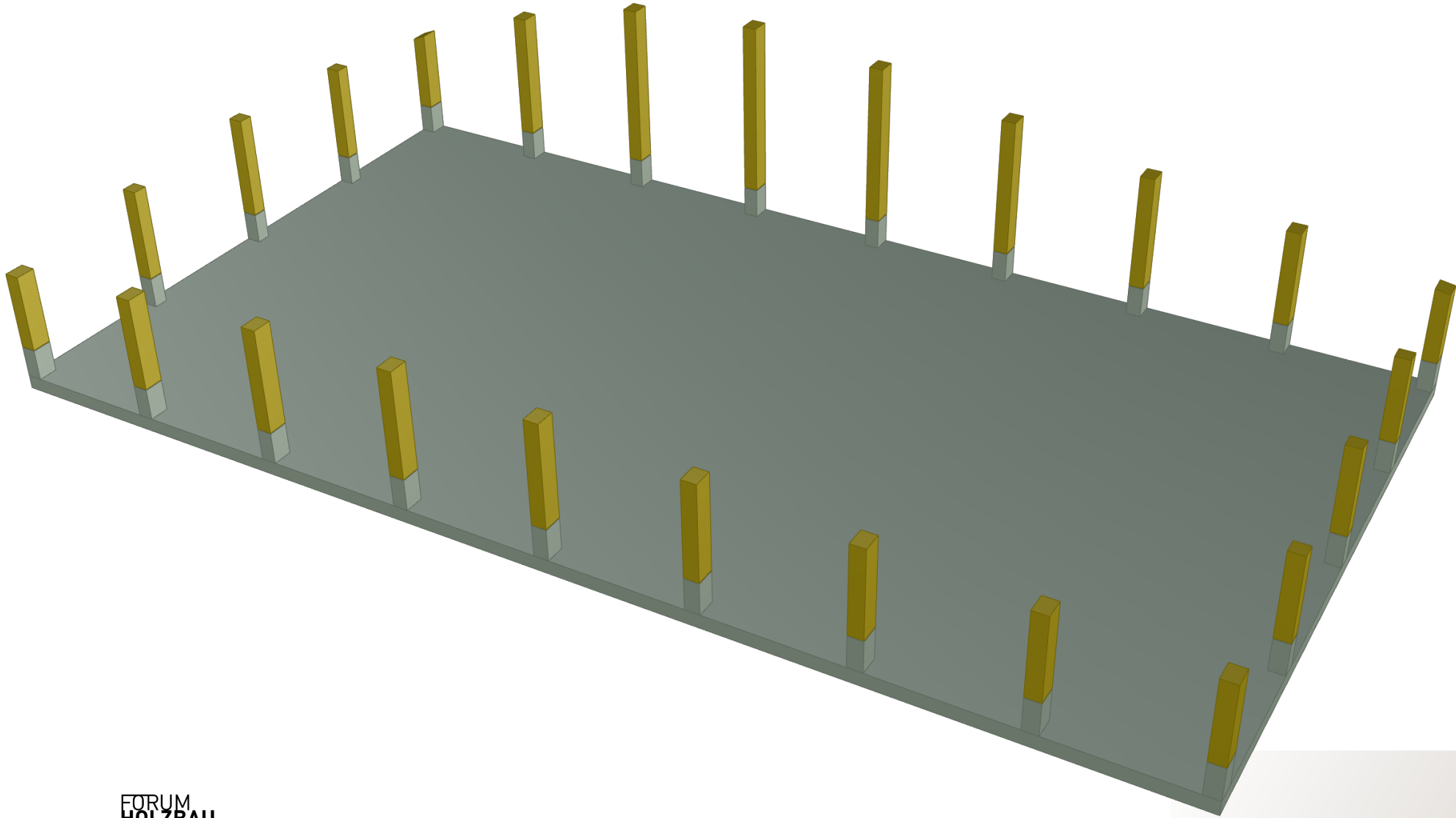
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Das Dach...



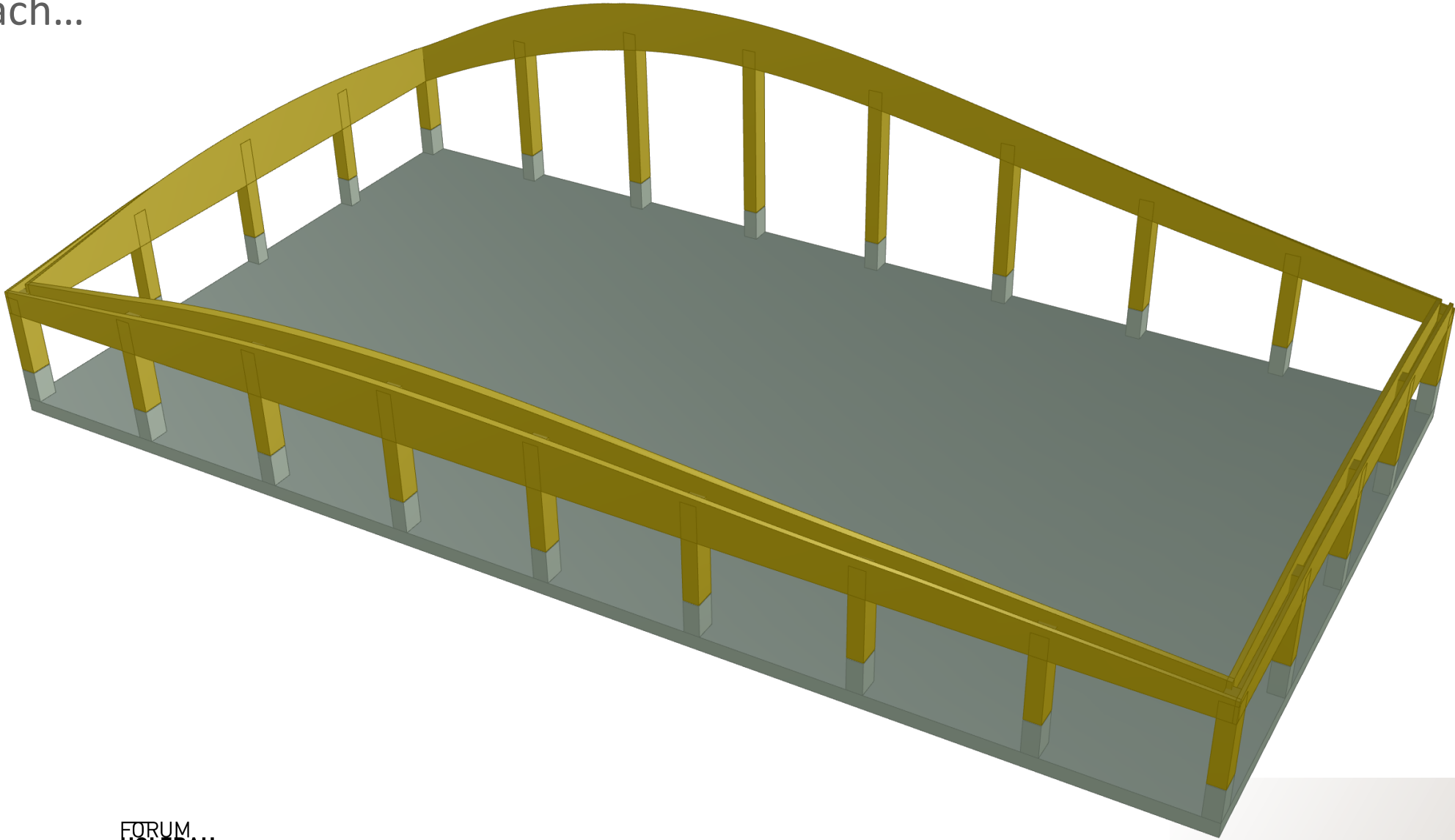
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Das Dach...



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Das Dach...



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Das Dach...



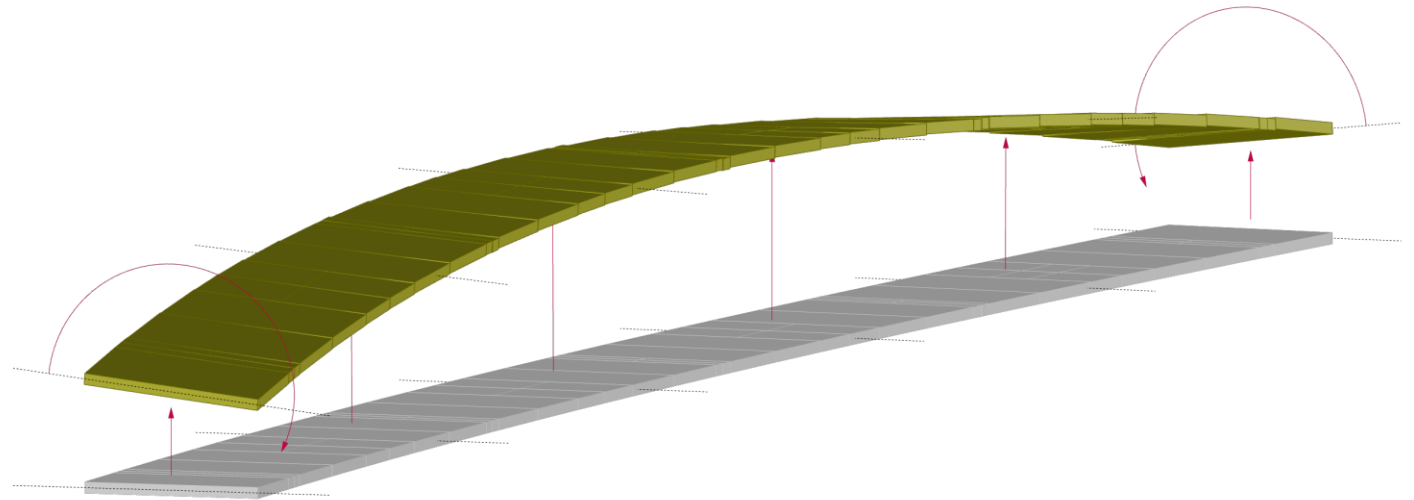
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Das Konzept – Wir machen folgendes Prinzip...

FSH Lamellen...



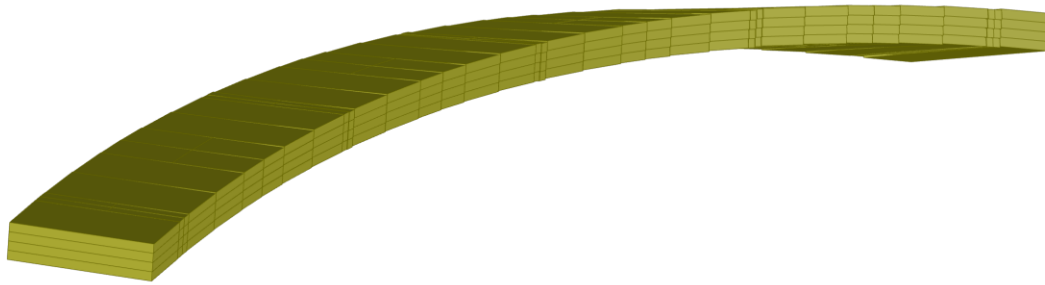
In Form biegen und tordieren...



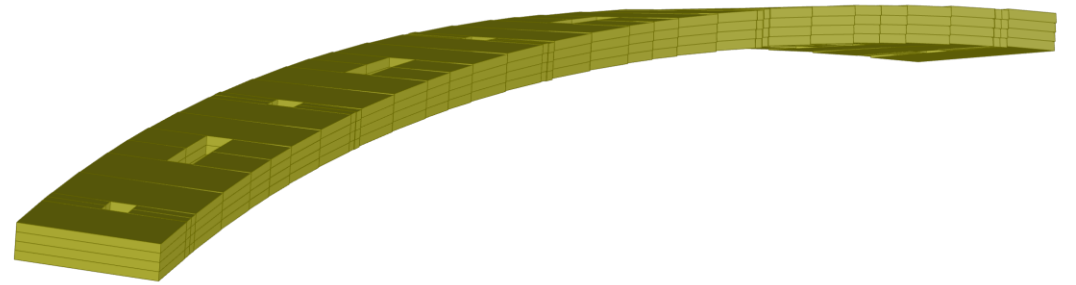
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Das Konzept – Wir machen folgendes Prinzip...

Lamellen aufeinander stapeln...



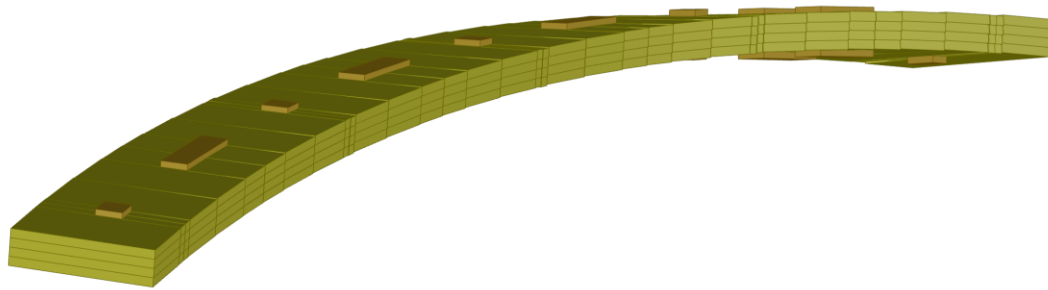
In die CNC-Fräse und Ausschnitte fräsen...



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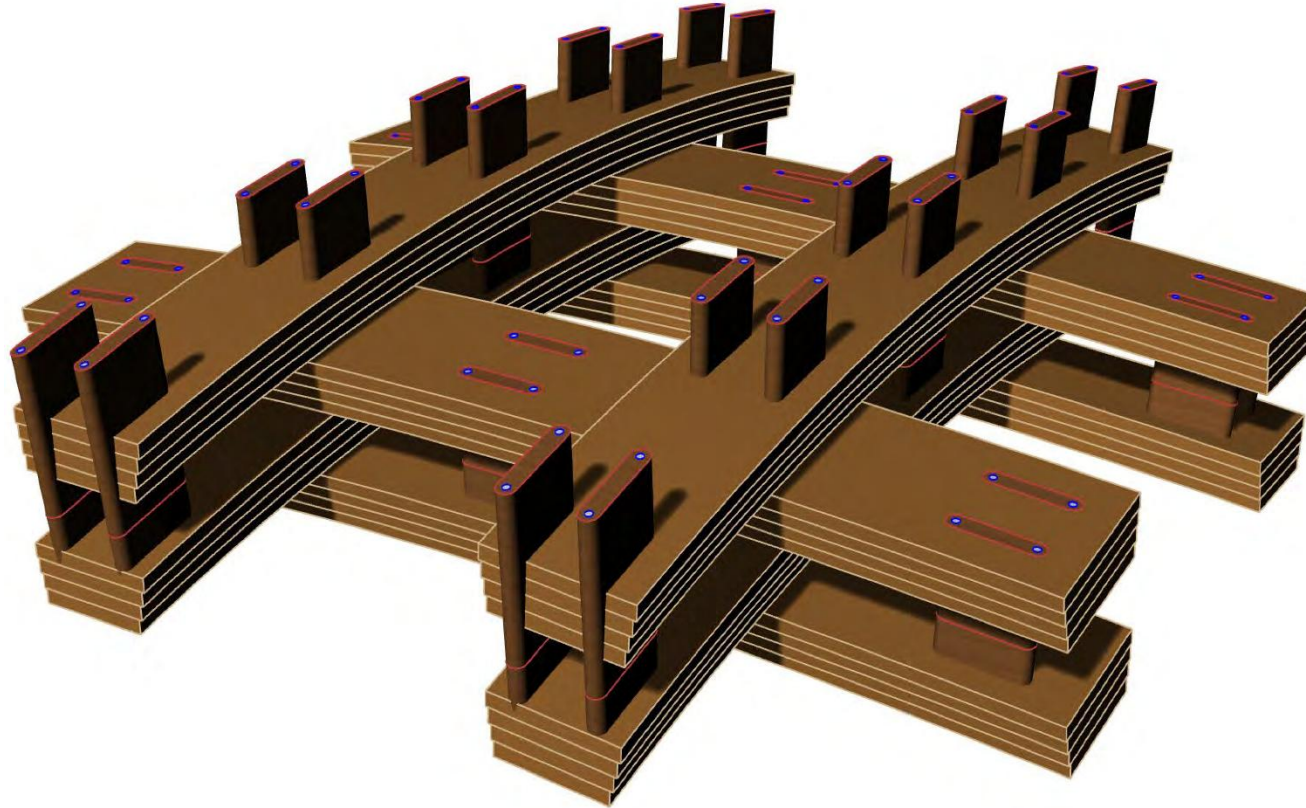
Das Konzept – Wir machen folgendes Prinzip...

Dübel einstecken...



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Das Konzept – Wir machen folgendes Prinzip...



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Die ersten zwei (drei) Mock-ups...



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Die ersten zwei (drei) Mock-ups...



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Die ersten zwei (drei) Mock-ups...

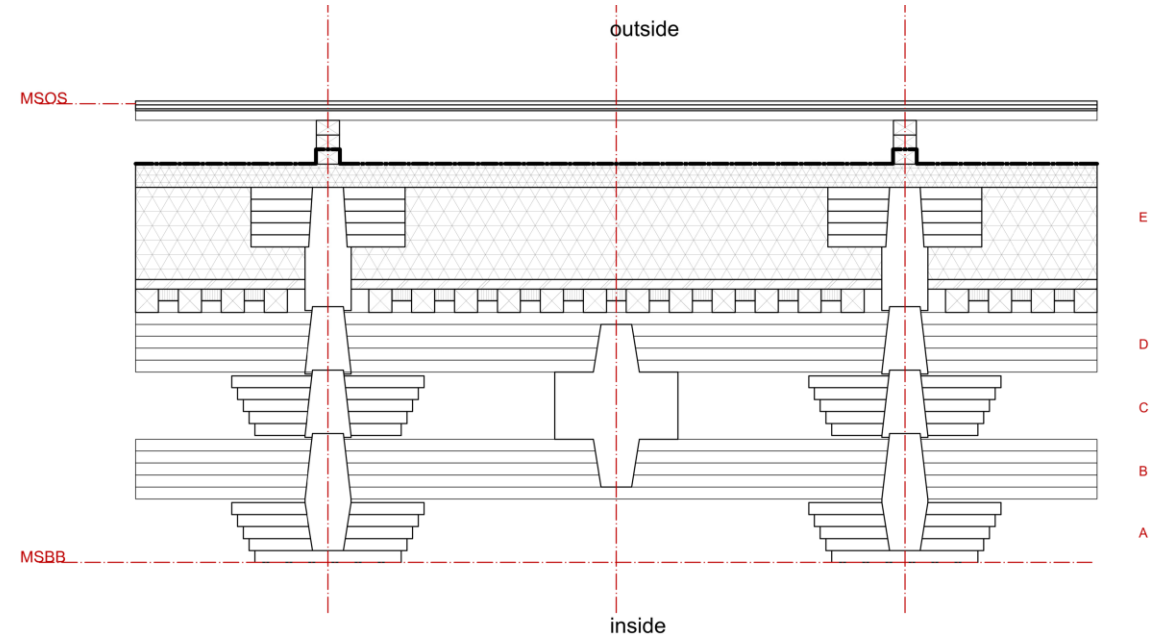
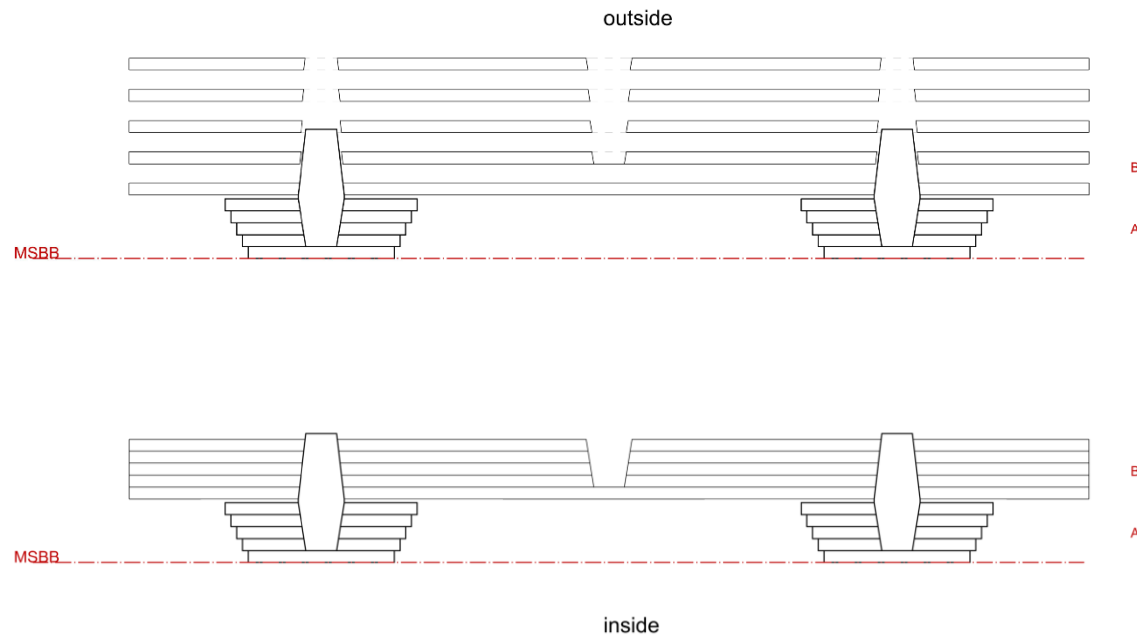
Was haben wir gelernt?

- Die Rückstellung ist zu gross, dadurch wird eine exakte Geometrie schwierig
- Wir hätten gerne dünnere Lamellen
- Zu viel Schlupf in den Verbindungen
- Abgestufte Lamellen sind gut
- Prinzip Dübel ist gut, aber noch nicht ganz ausgereift
- ...

→ **Konzept!**

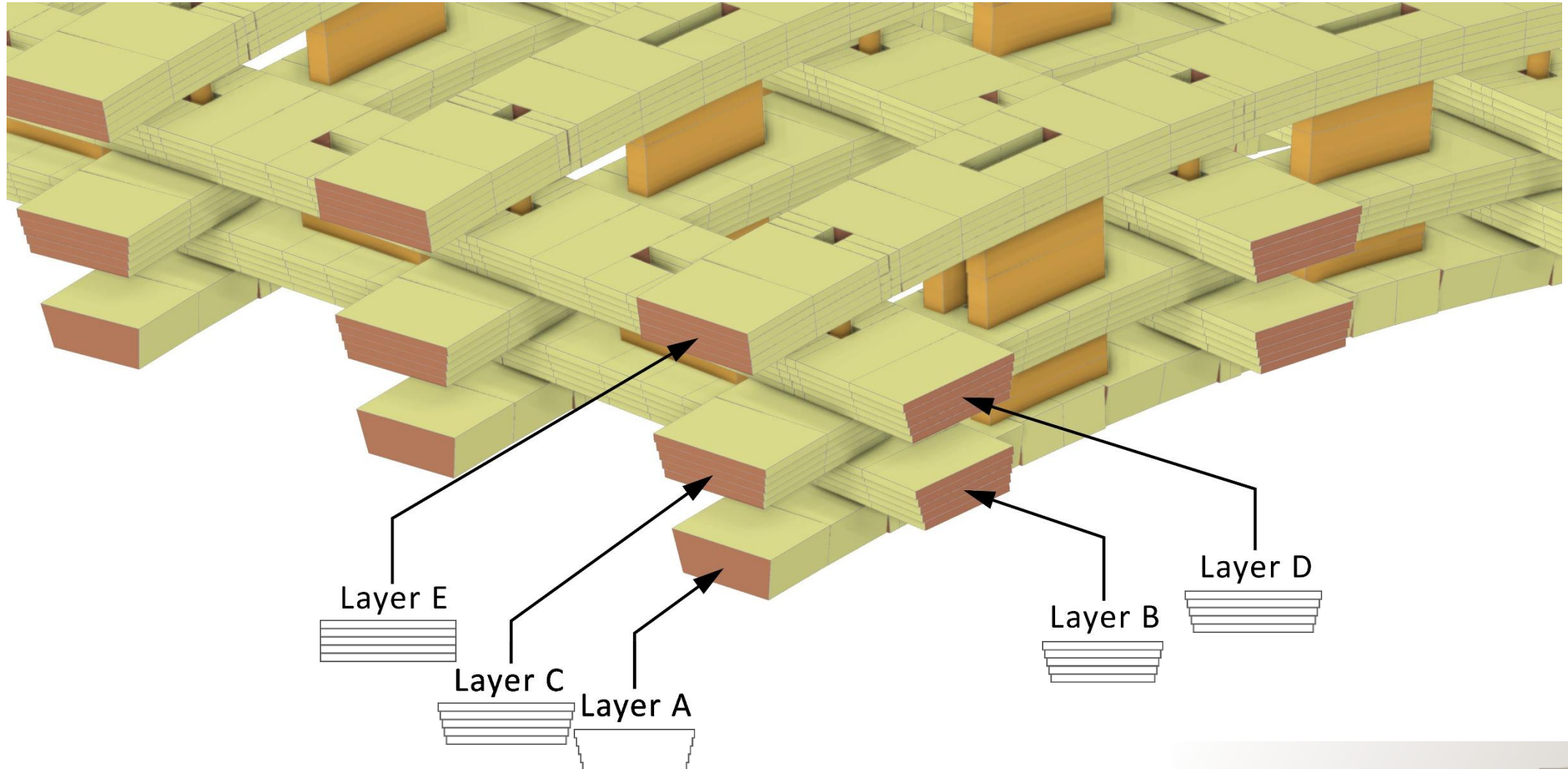
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Das Konzept



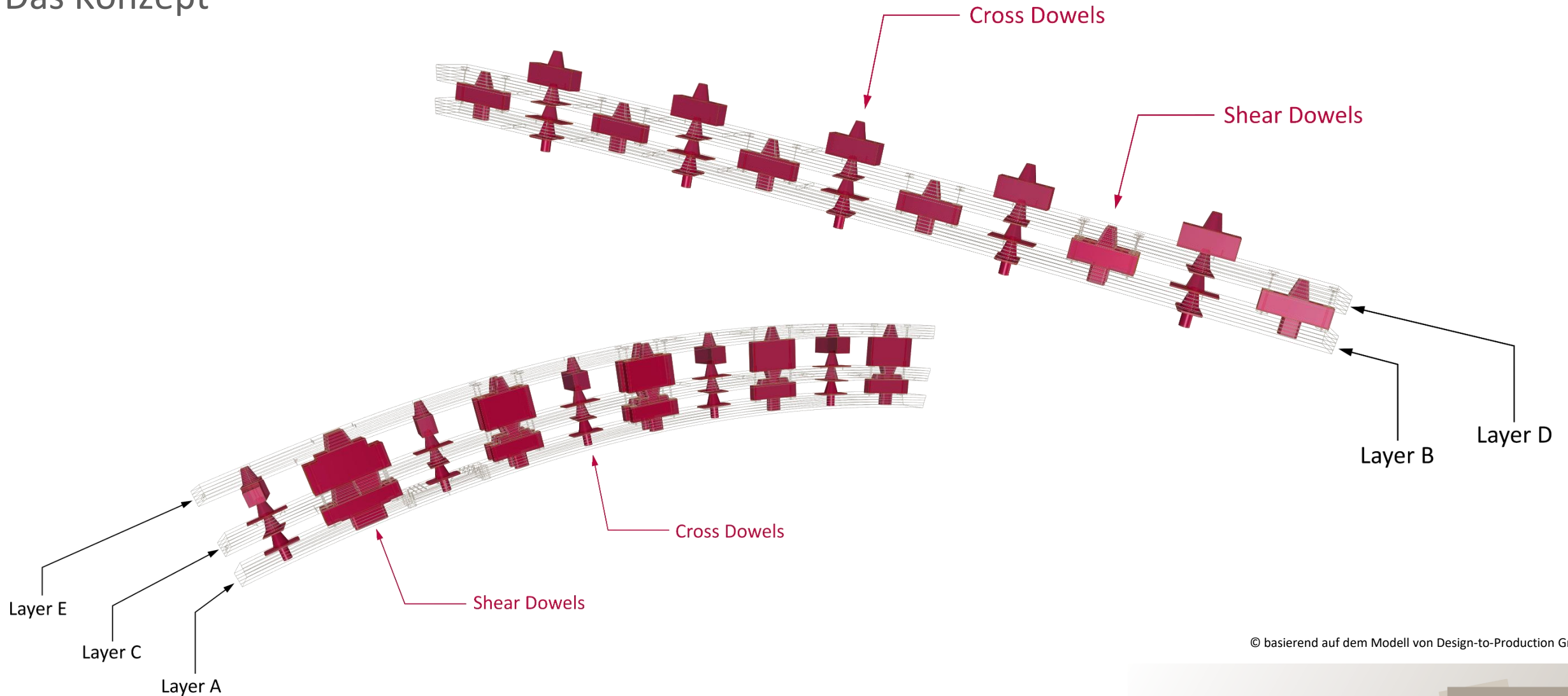
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Das Konzept



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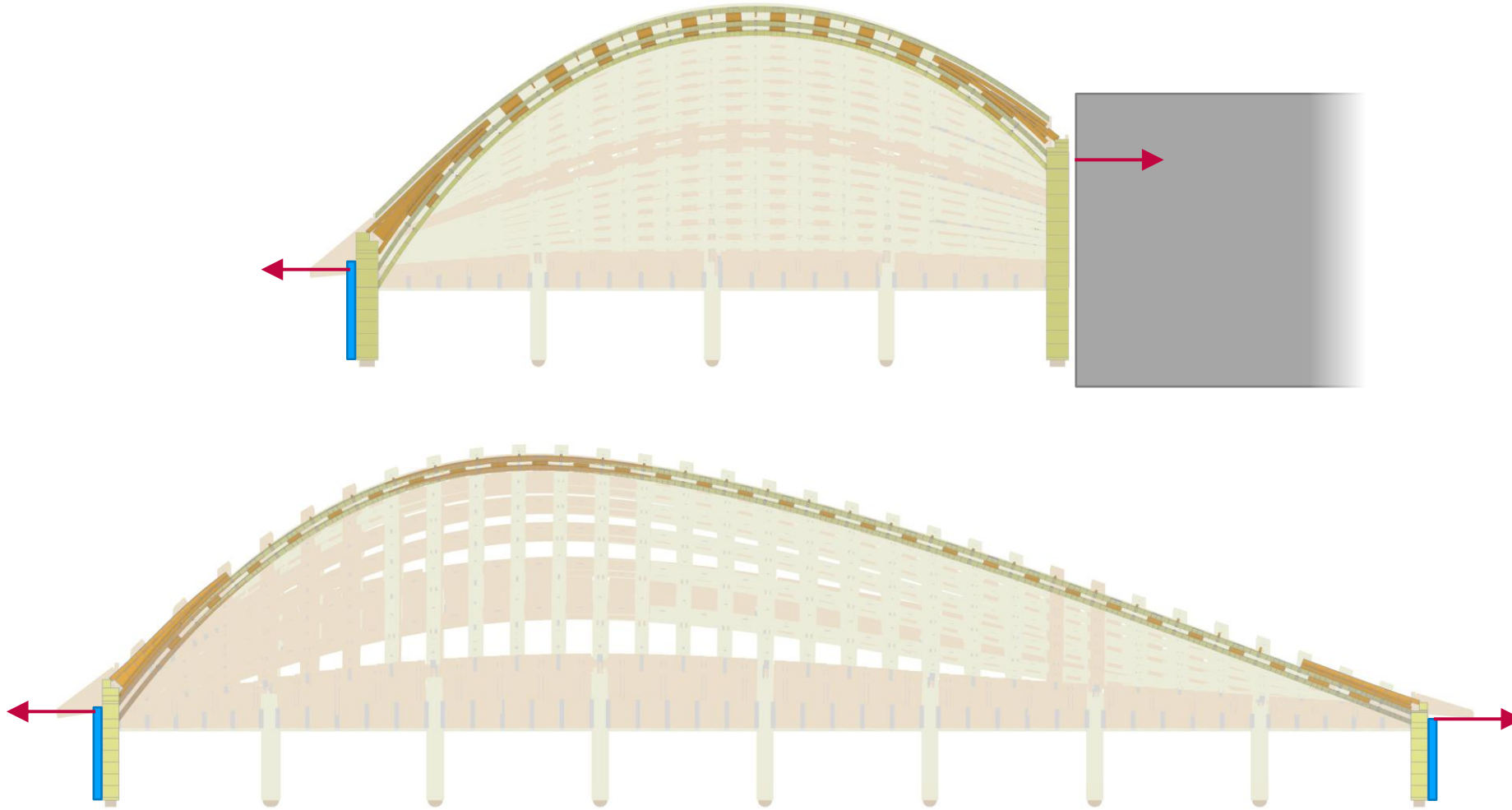
Das Konzept



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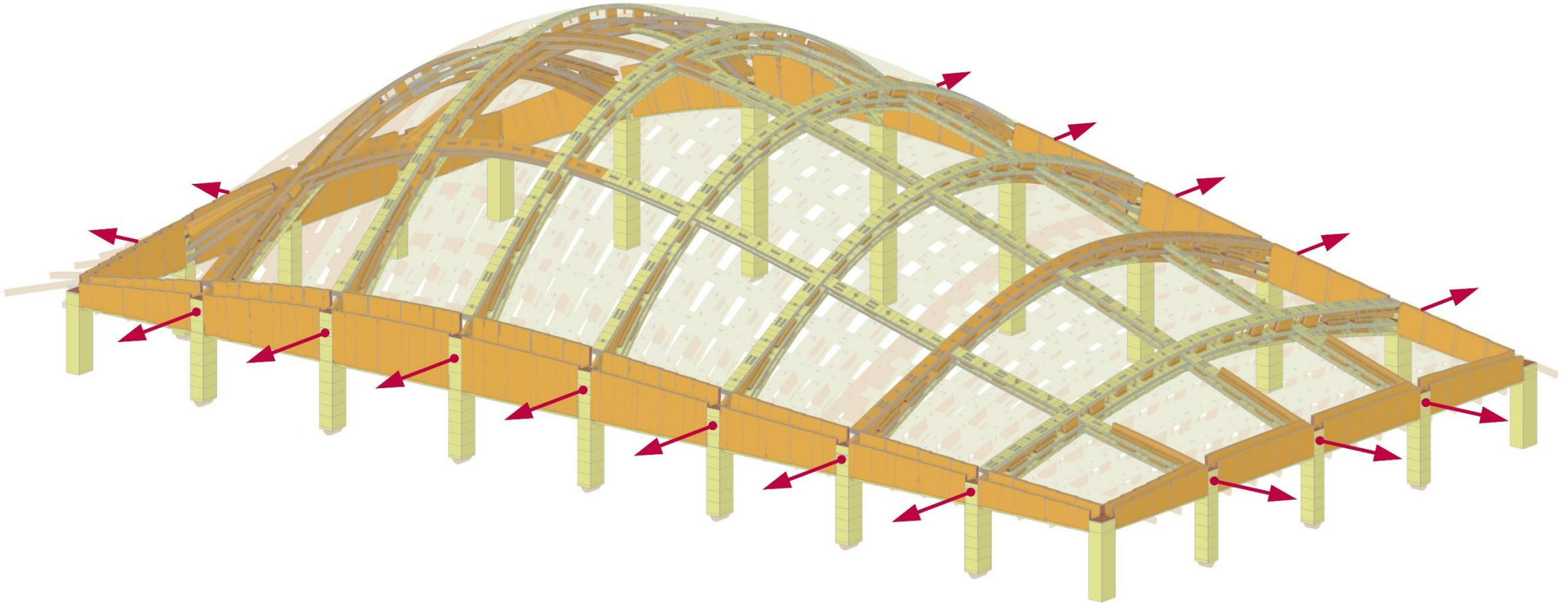
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Die Dachgeometrie



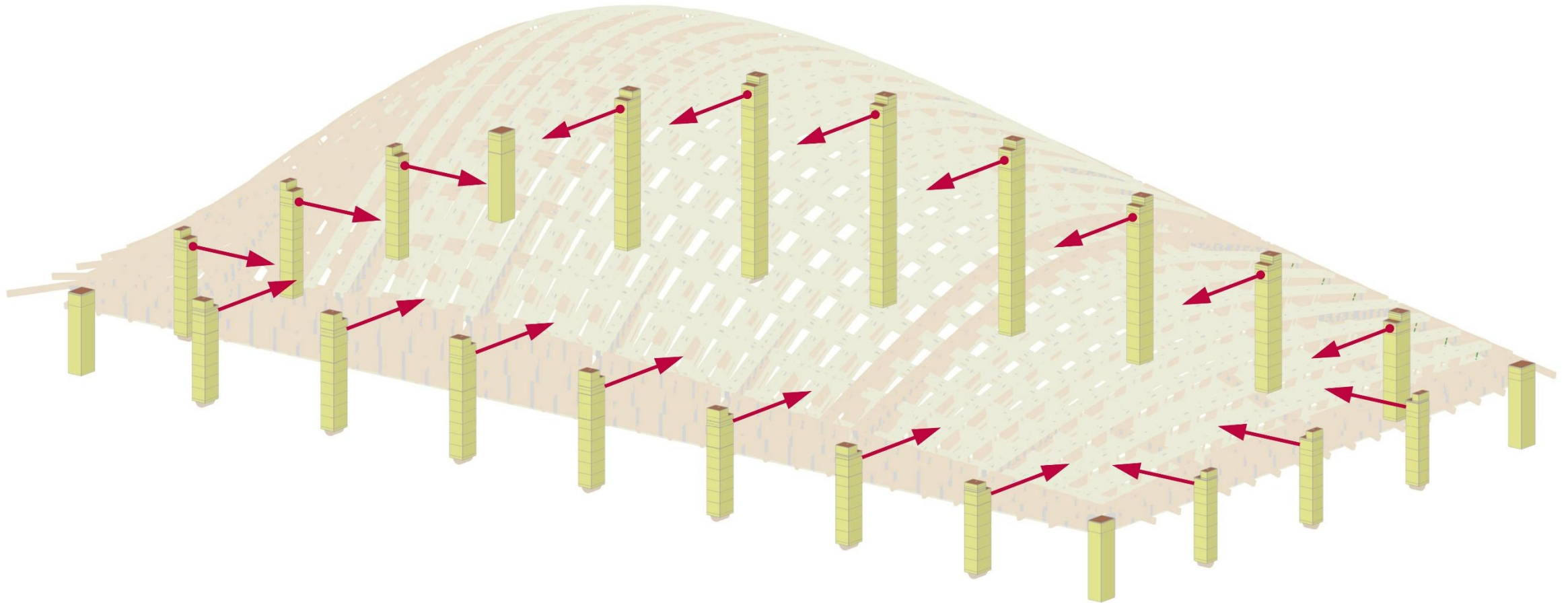
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Die Dachgeometrie



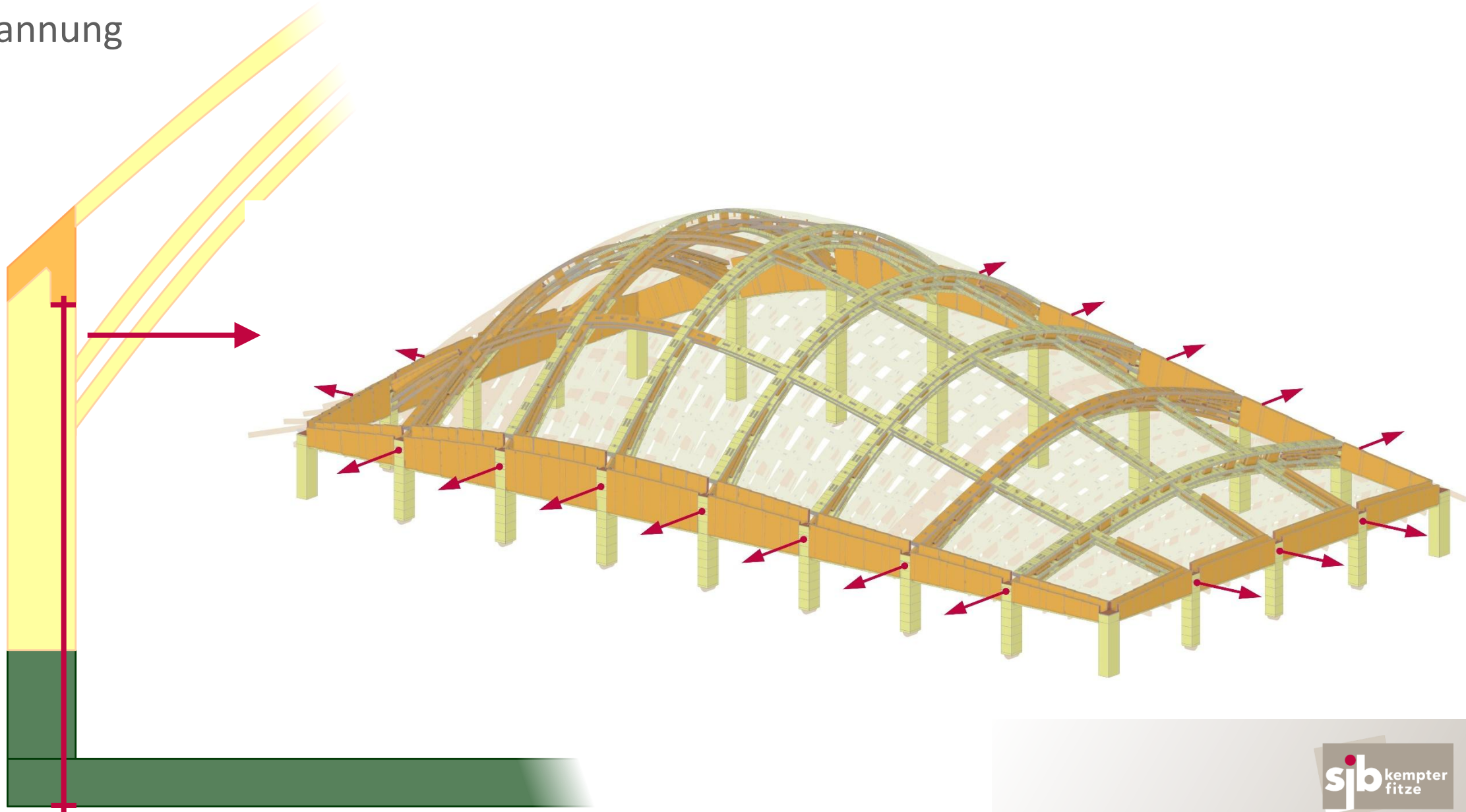
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Die Dachgeometrie



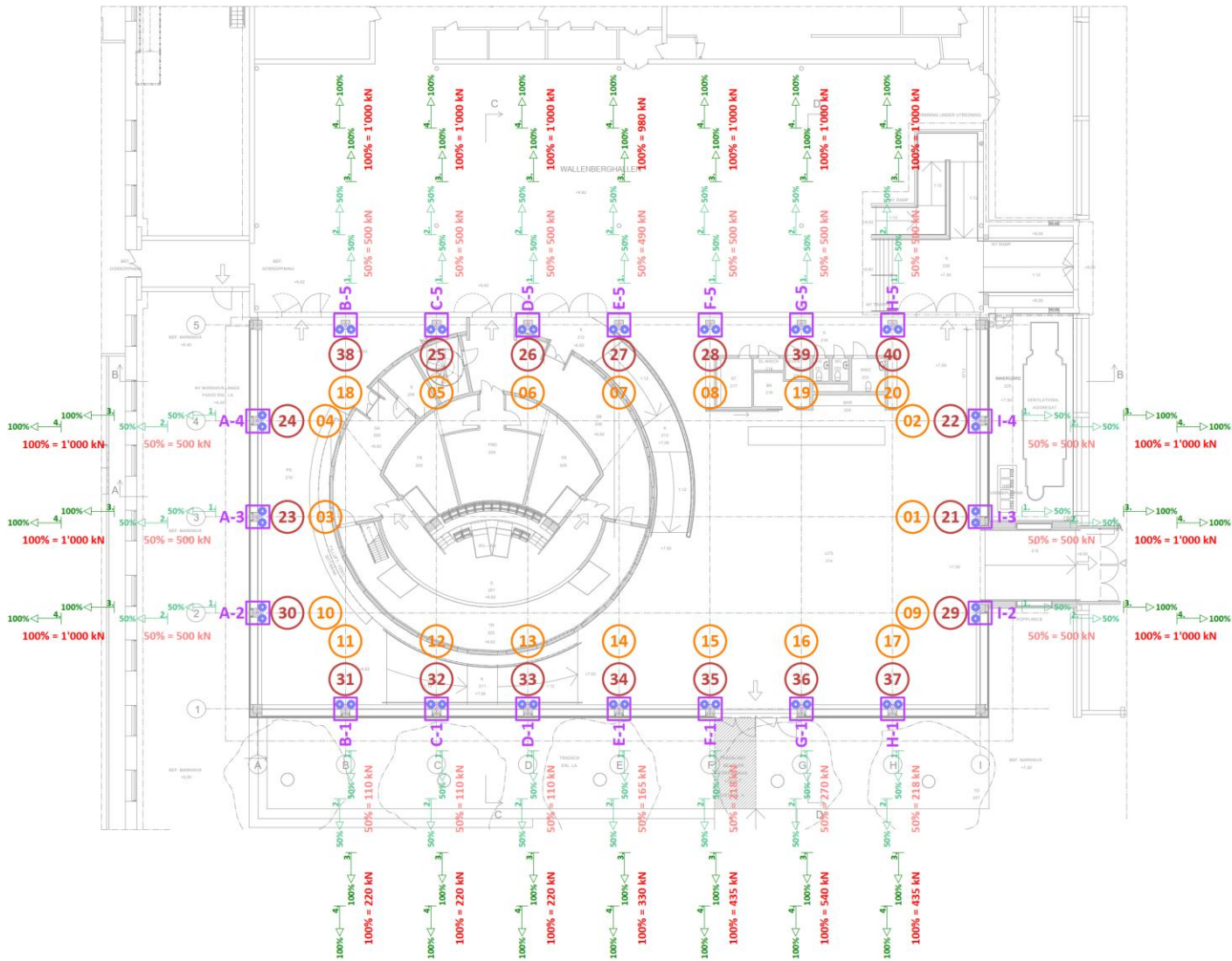
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Die Vorspannung



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Die Vorspannung

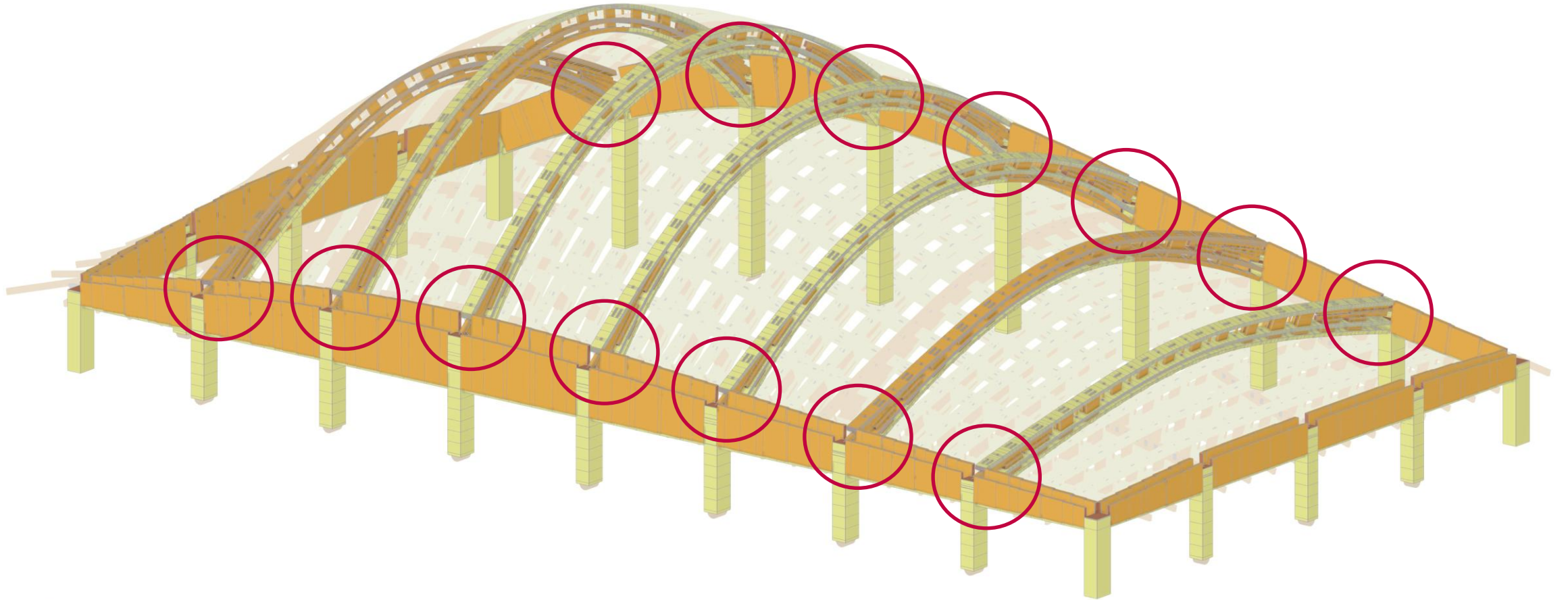


01	pre-tension sequence ROUND 1
21	pre-tension sequence ROUND 2
I-4	No. column
100% = 1'000 kN	pre-tension stages ROUND 1
50% = 500 kN	pre-tension stages ROUND 2
100% = 1'000 kN	pre-tension force per rod ROUND 1
50% = 500 kN	pre-tension force per rod ROUND 2
0	pre-tension rods

No. column	final pre-tension force per rod
A-5	- (per rod)
B-5	1'000 kN (per rod)
C-5	1'000 kN (per rod)
D-5	1'000 kN (per rod)
E-5	980 kN (per rod)
F-5	1'000 kN (per rod)
G-5	1'000 kN (per rod)
H-5	1'000 kN (per rod)
I-5	- (per rod)
I-4	1'000 kN (per rod)
I-3	1'000 kN (per rod)
I-2	1'000 kN (per rod)
I-1	- (per rod)
H-1	435 kN (per rod)
G-1	540 kN (per rod)
F-1	435 kN (per rod)
E-1	330 kN (per rod)
D-1	220 kN (per rod)
C-1	220 kN (per rod)
B-1	220 kN (per rod)
A-1	- (per rod)
A-2	1'000 kN (per rod)
A-3	1'000 kN (per rod)
A-4	1'000 kN (per rod)

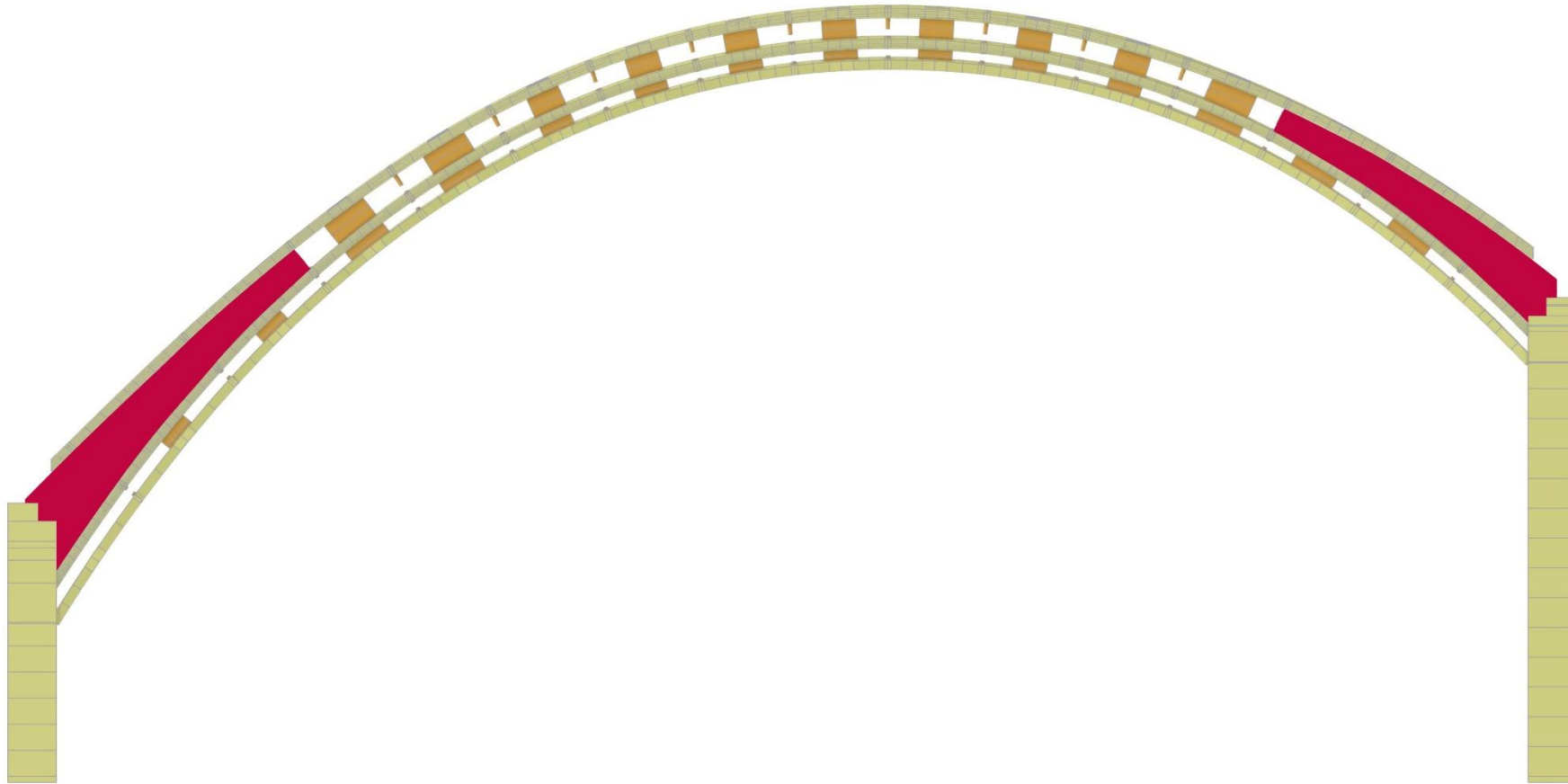
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Die Rahmenecke



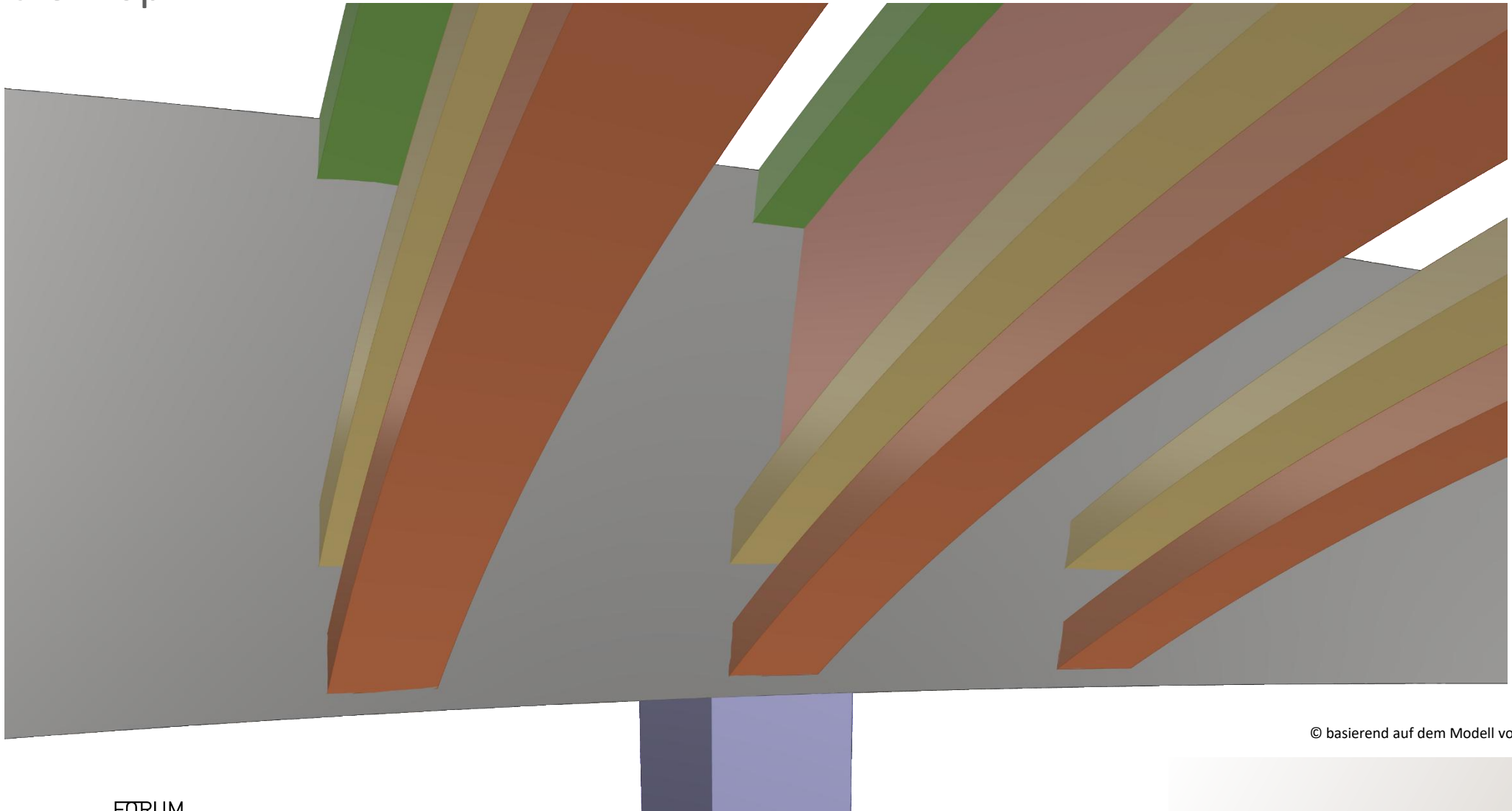
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Die Rahmenecke



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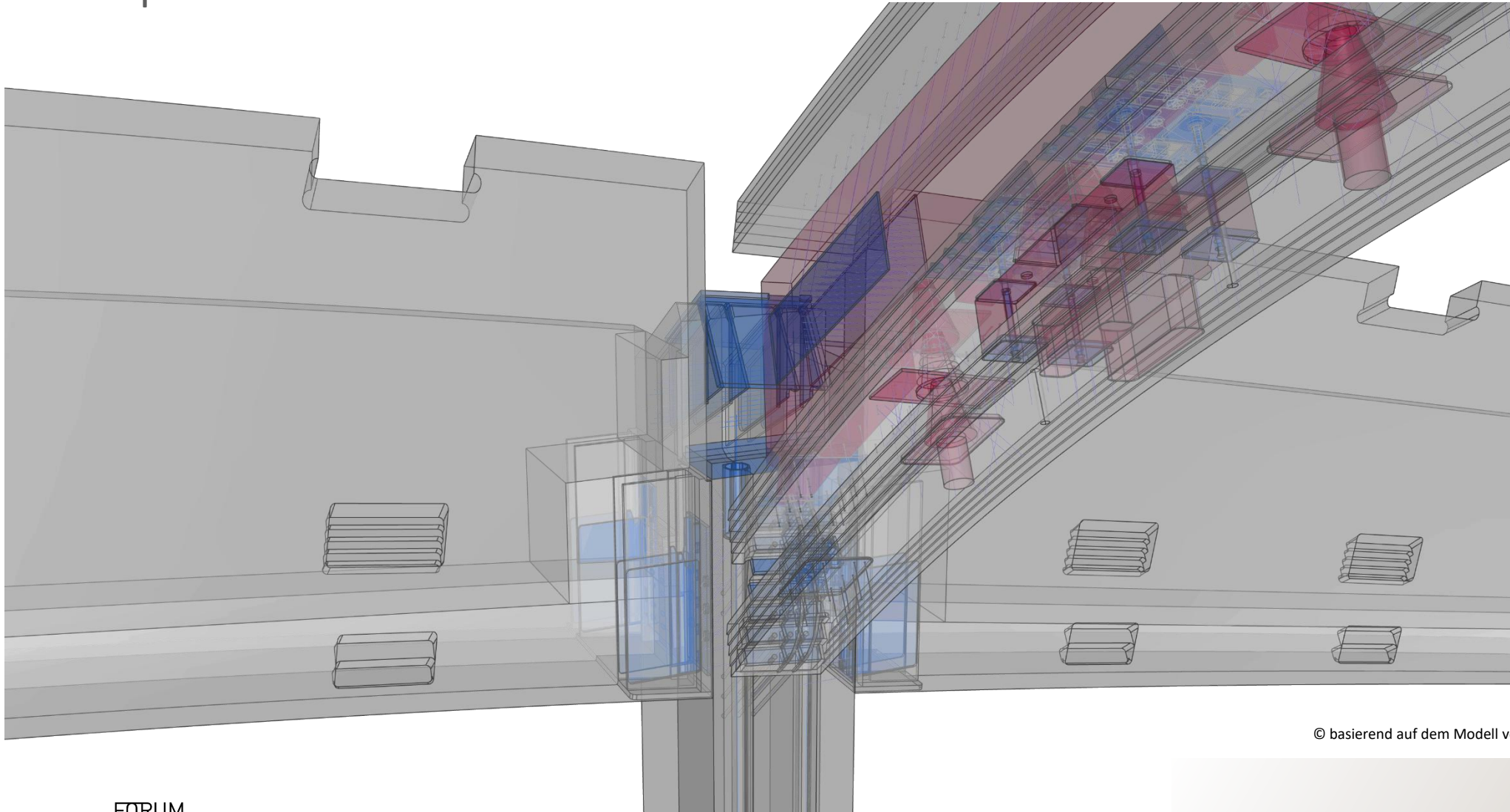
Der Stützenkopf



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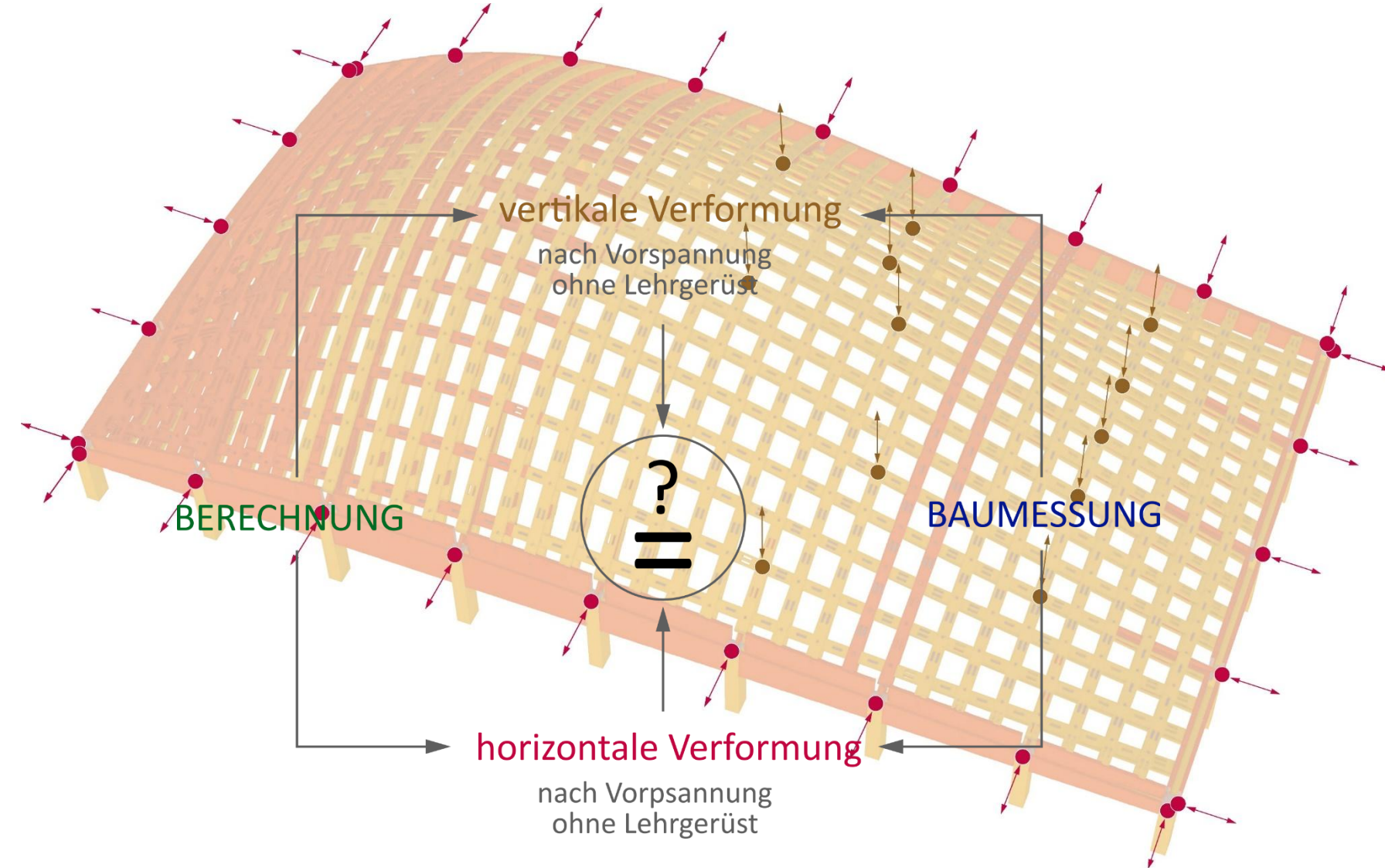
Der Stützenkopf



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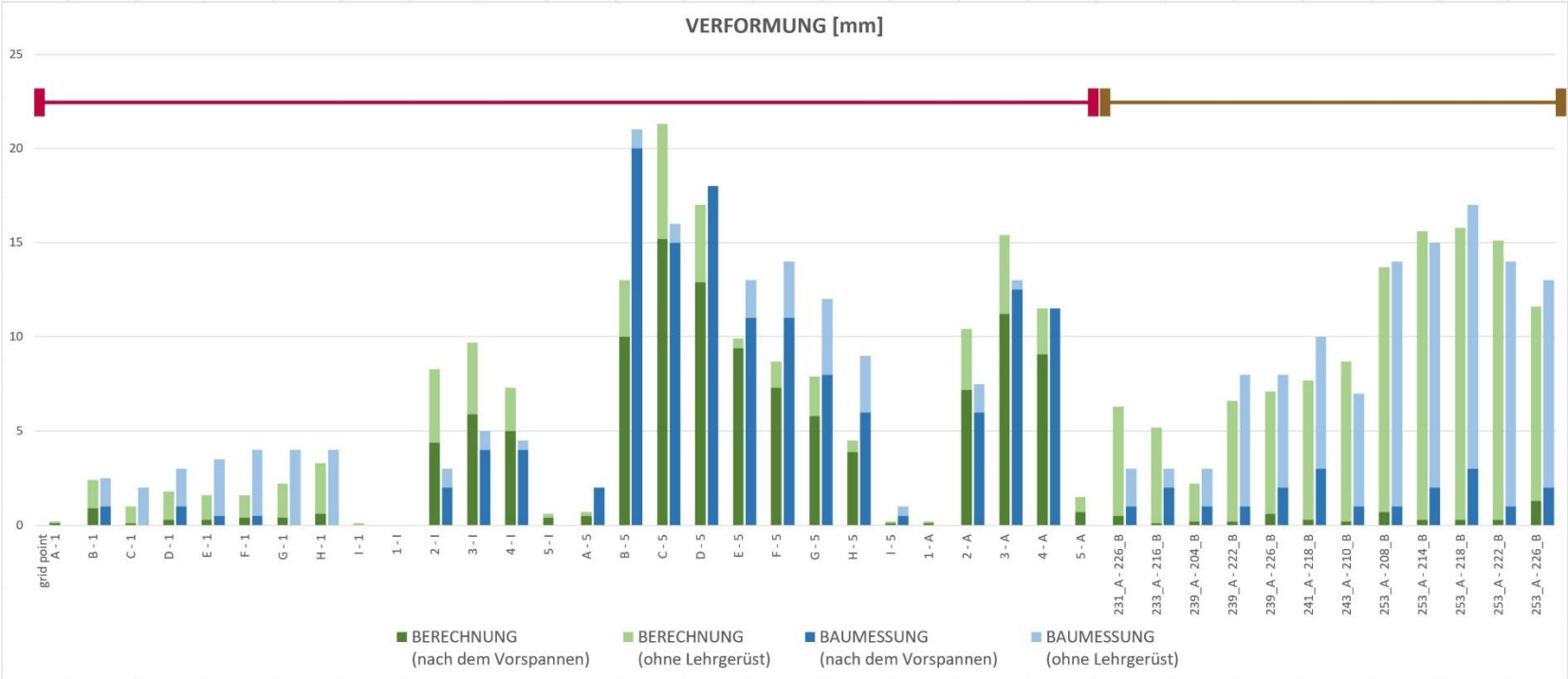
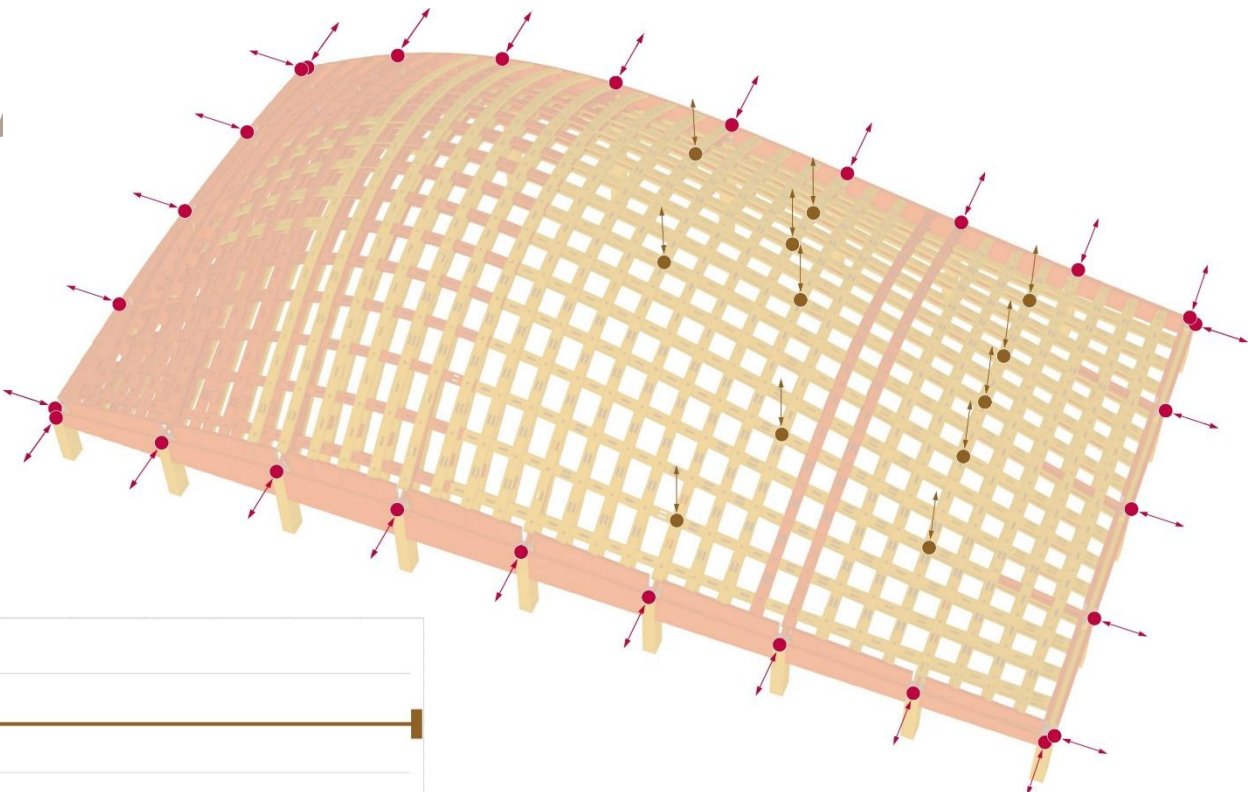
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Das statische Modell



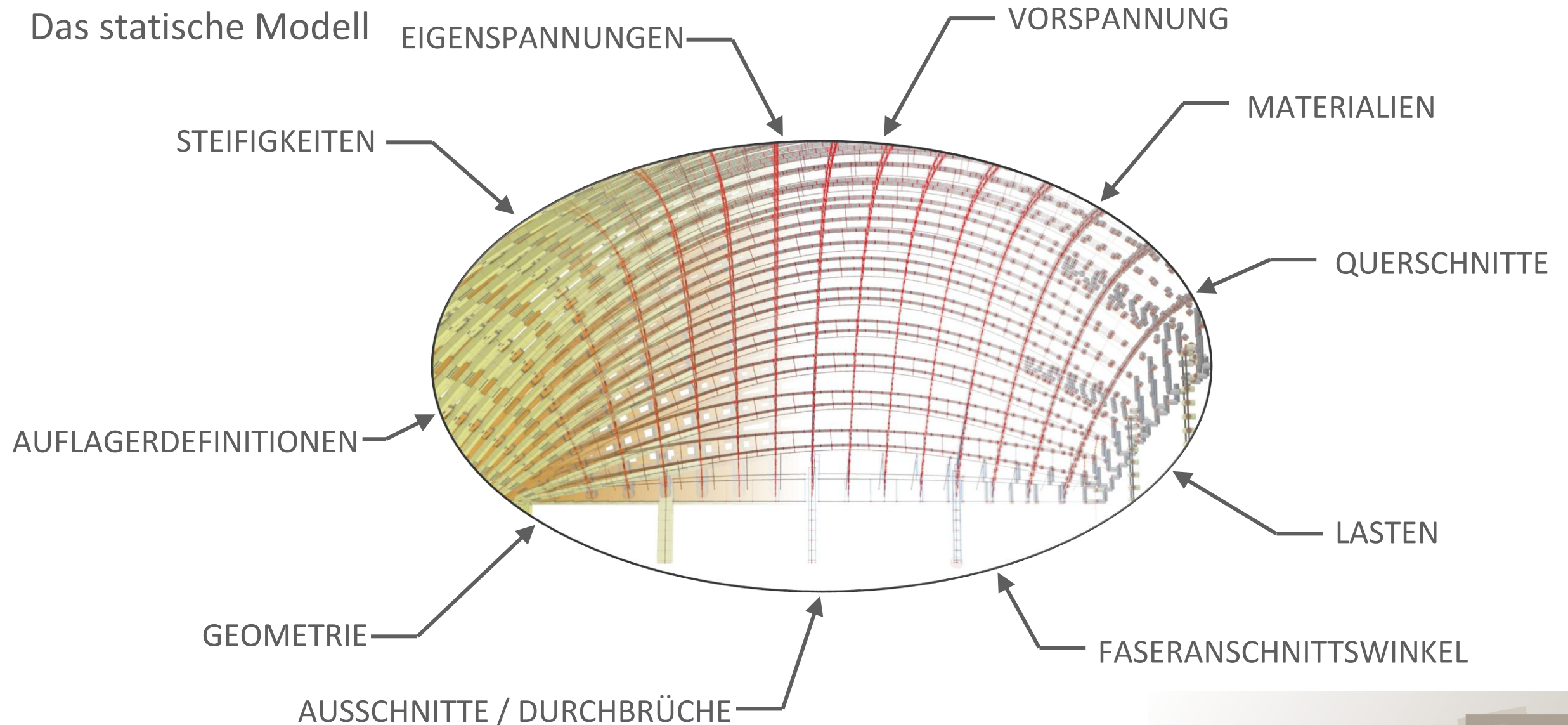
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Das statische Modell



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Das statische Modell



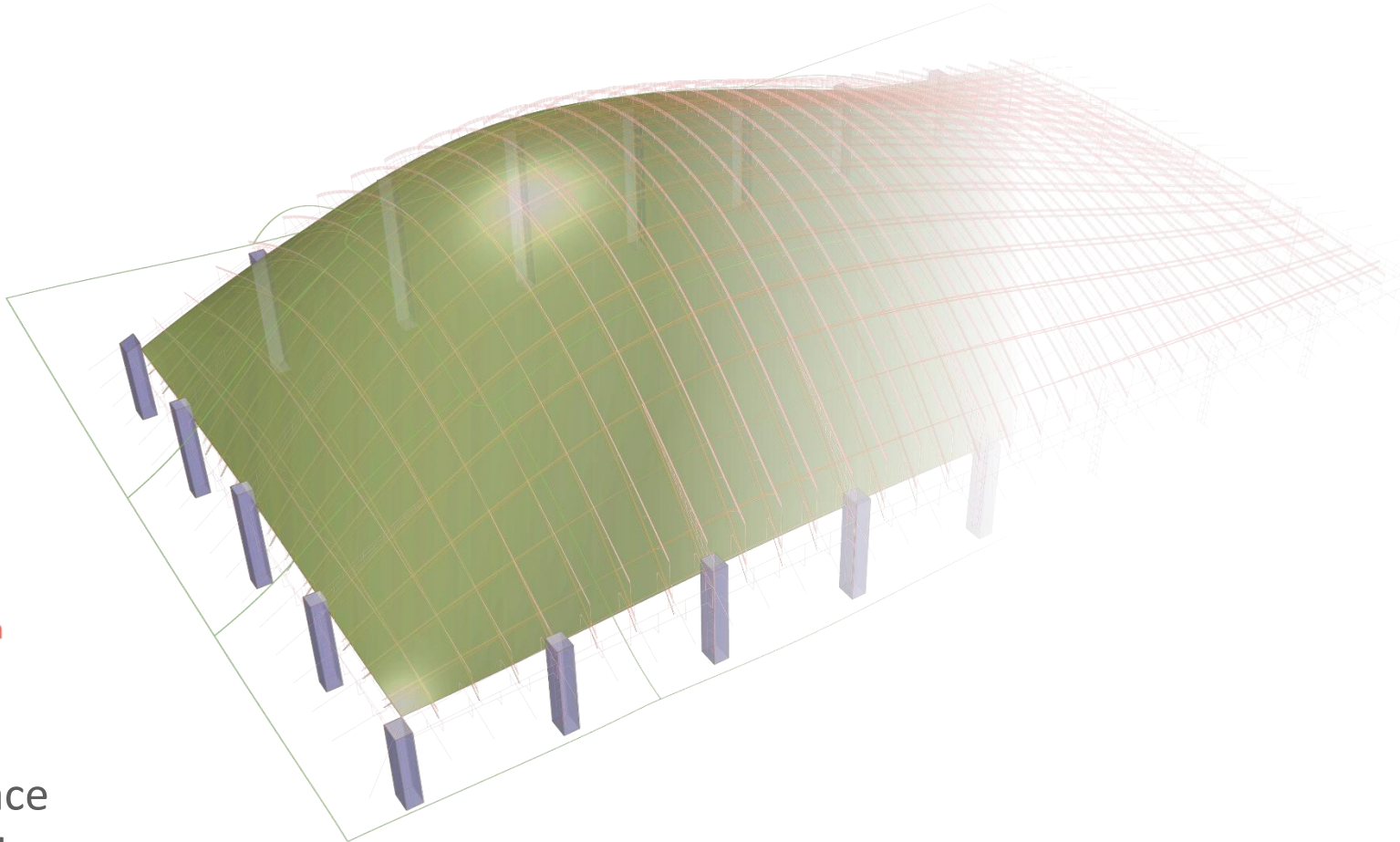
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Das statische Modell

INPUT



- Stützen
- Mastersurface
- Achsen der Lagen



OUTPUT



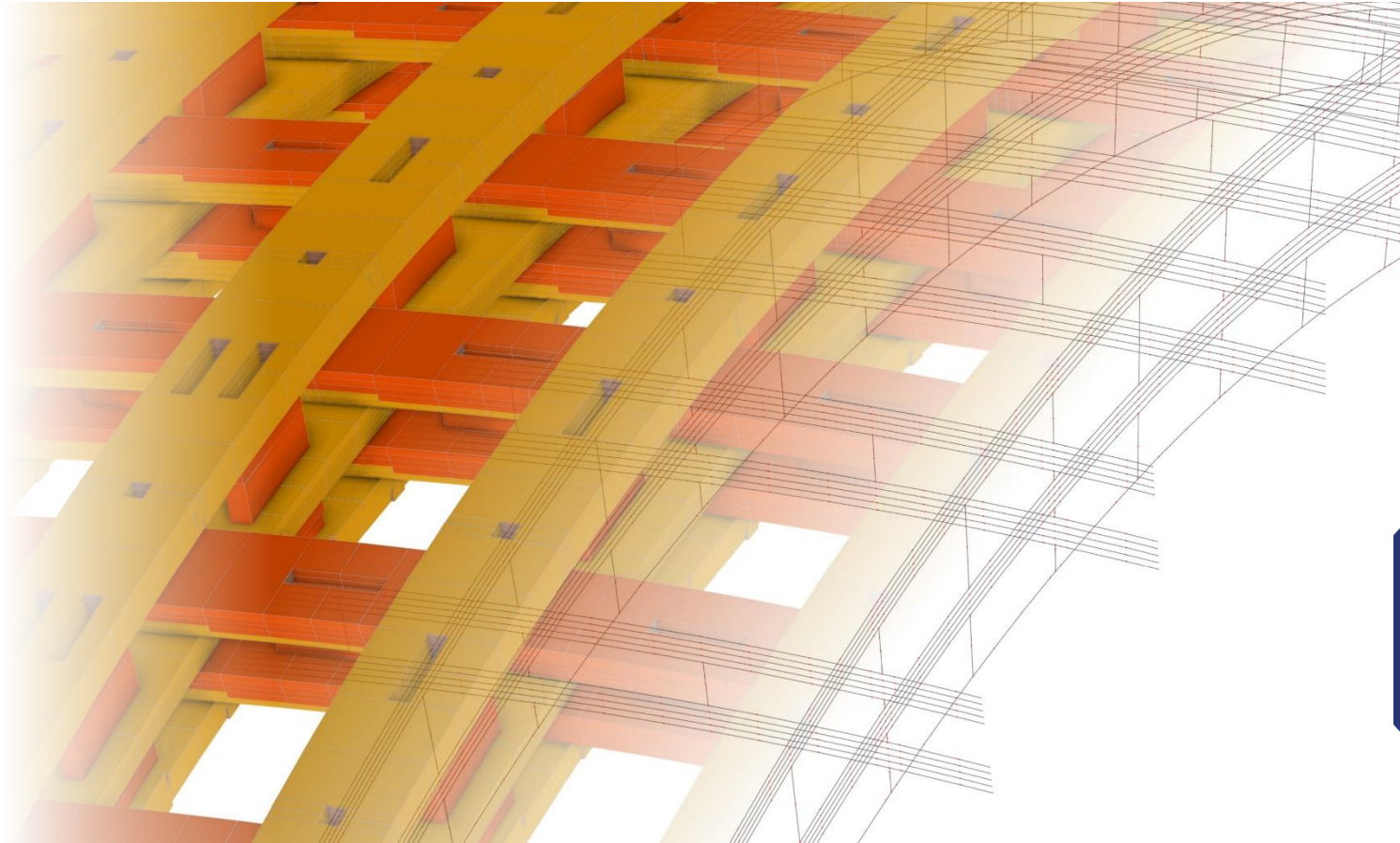
- alle Stäbe
- Querschnitte
- Materialien
- Stabdrehungen
- Kopplungsstäbe
- ...



- 118'557 Knoten
- 153'746 Stäbe
- 941 Querschnitte

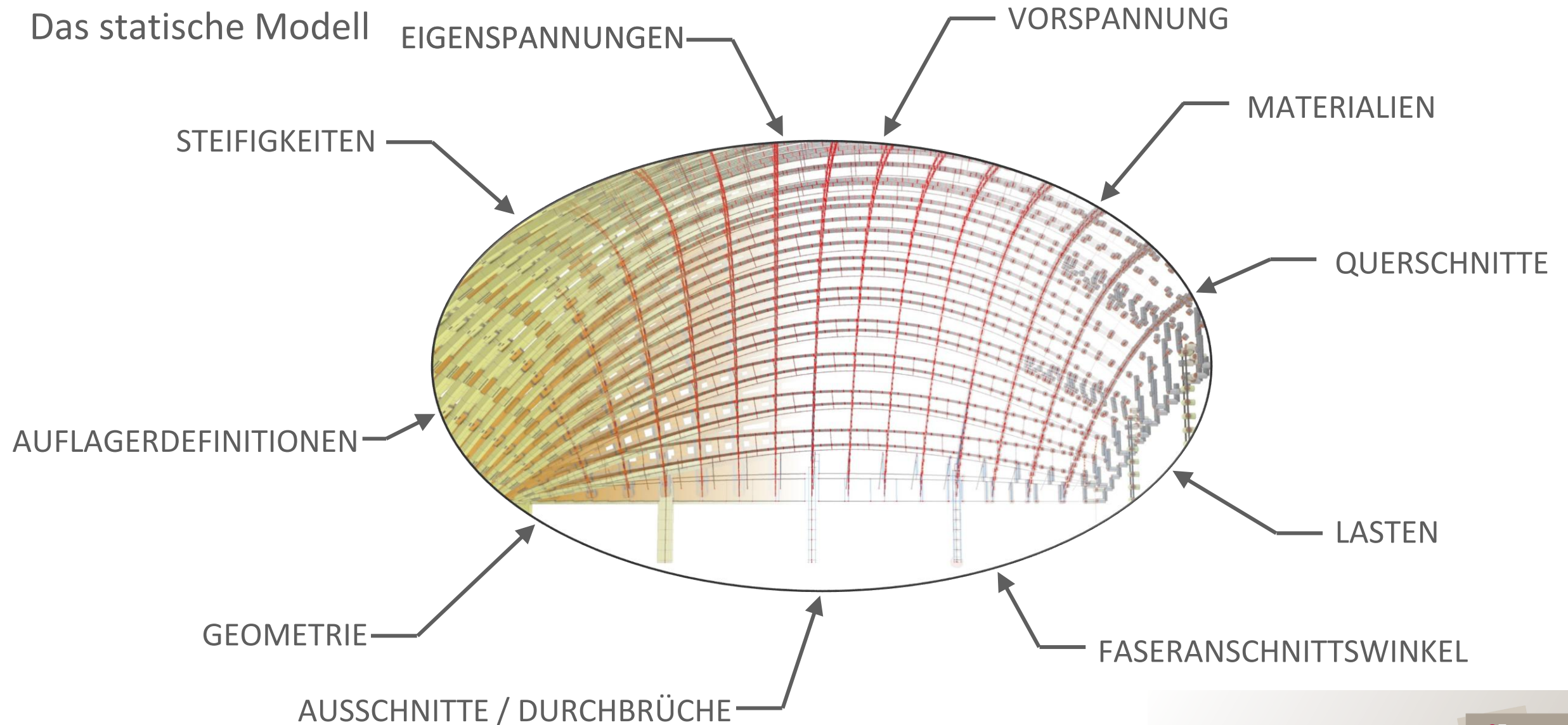
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Das statische Modell



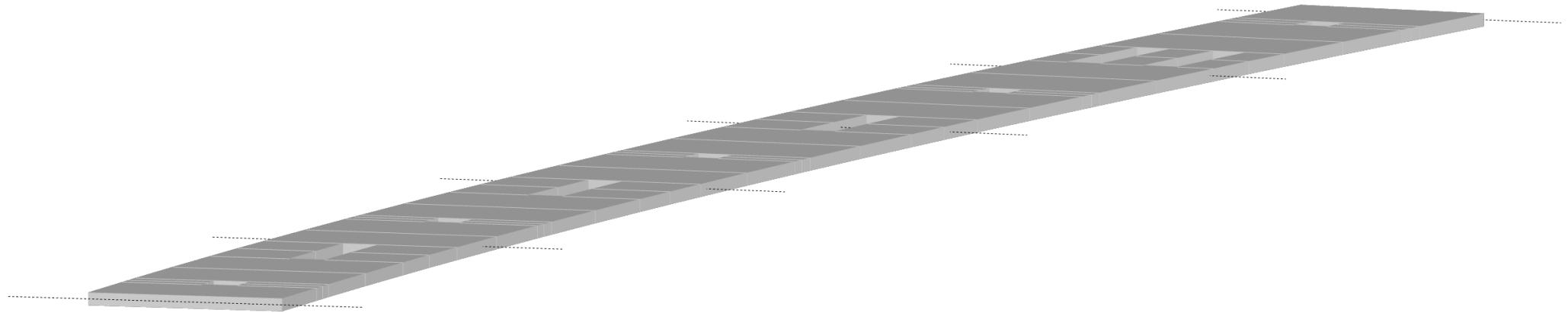
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Das statische Modell



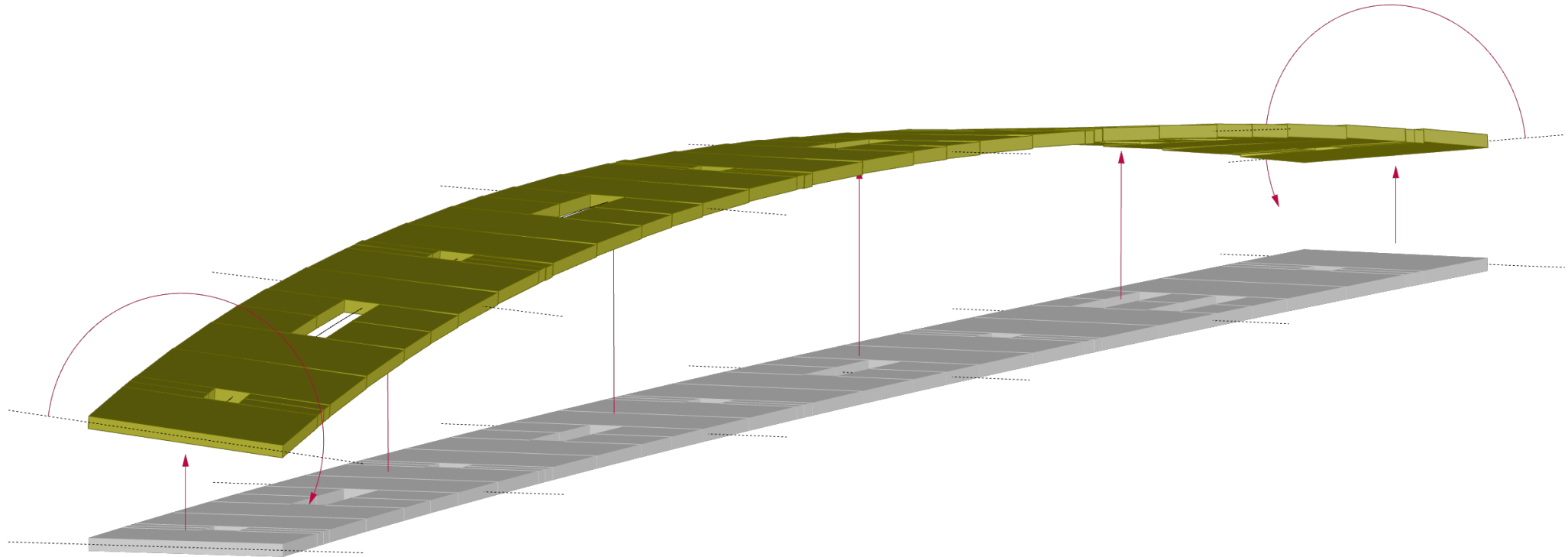
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Die Eigenspannungen



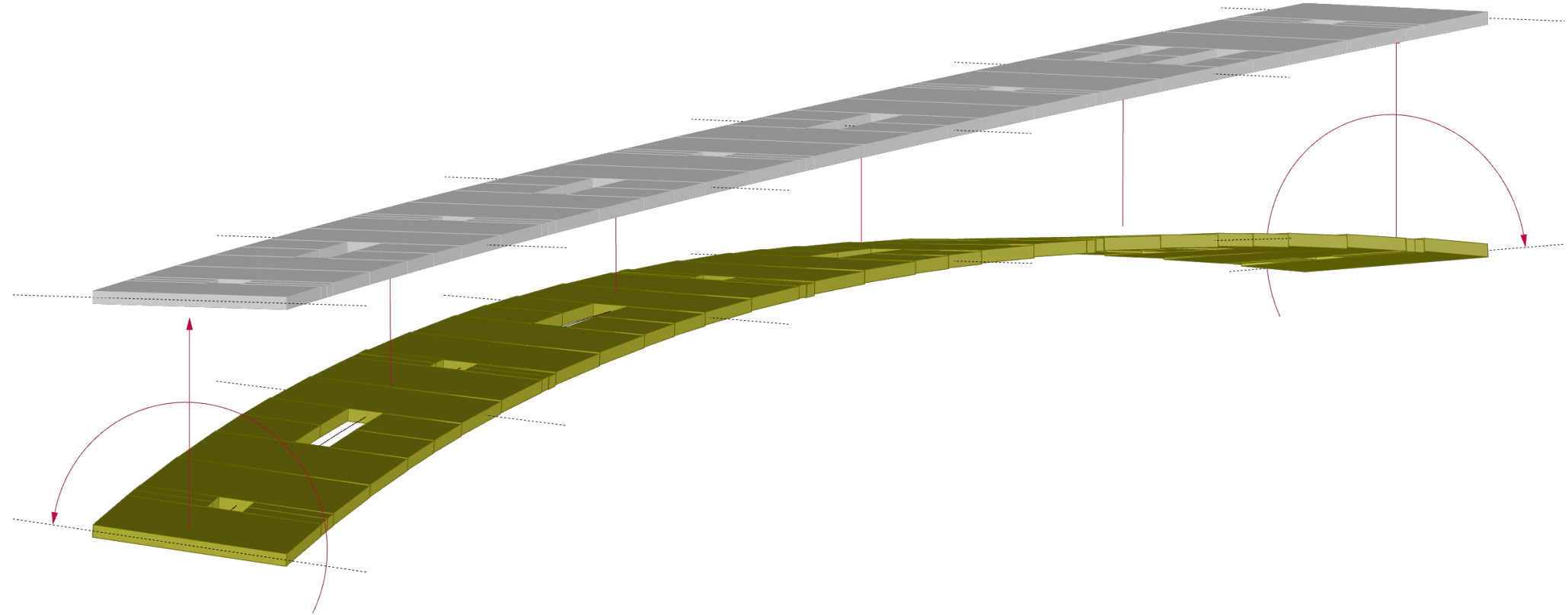
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Die Eigenspannungen



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Die Eigenspannungen



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Die Redundanz

Wisdom Stockholm, Sweden

9 Sensitivity Analysis

9.1 General

Sensitivity Analysis:	- Because of the dependence of the structure to the stiffness distribut done to evaluate the influences of stiffness variations to the structure
General:	- Worst case scenarios are also modelled to determine the impacts or at: failure of a high stressed shear dowel, failure of a high stresses ten - One exemplary load case is picked for the sensitivity analysis to evali

9.2 Scenarios

Sensitivity Analysis:	Scenario:	Structural Adjustments:
Scenarios:	00	base line, structural model without adjustr
	01	decrease of all E-Modules and member hini
	02	increase of all E-Modules and member hing
	03	decrease of all E-Modules of the timber stru
	04	decrease of all member hinges of the timbe
	05	decrease of the Shear Dowel member hinge
	06	slippage of the Shear Dowels of +/- 0.50mm
	07	failure of a high stressed Shear Dowel
	08	decrease of pre-tension force to 90%
	09	failure of a pair of high stressed pre-tension rods

Project No.: 821.062
Project: Wisdom, Stockholm
Title: sensitivity analysis (preliminary design)
Date: 13.10.2021
Author: sr

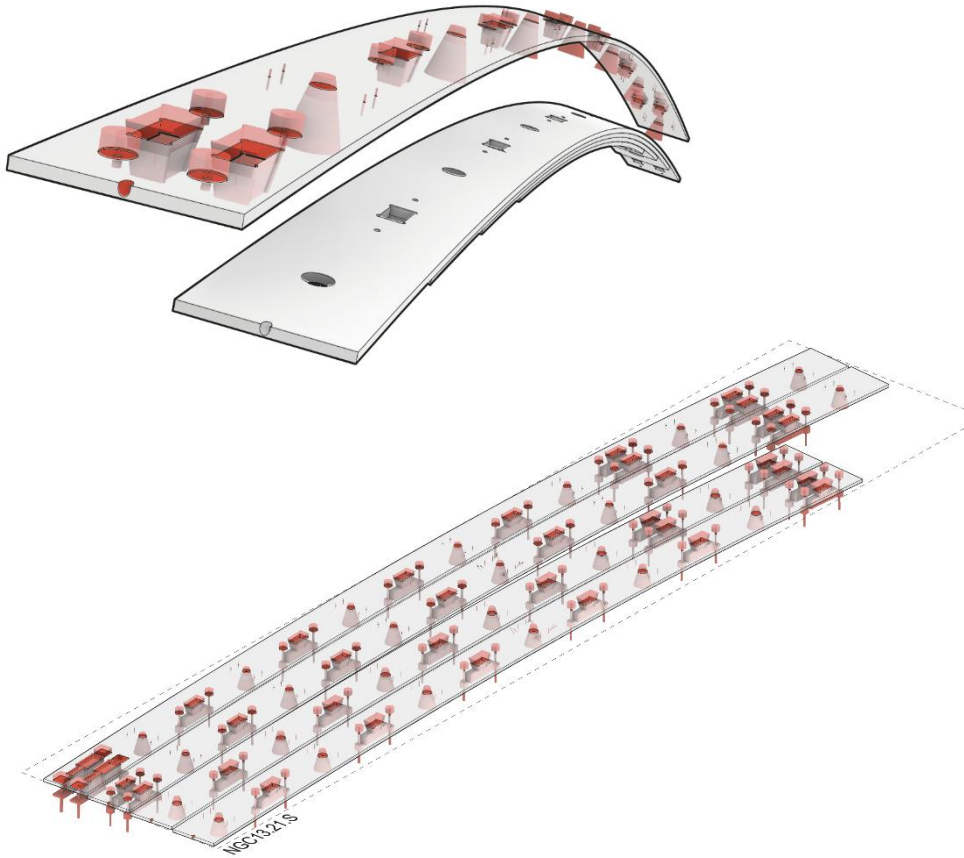
STRUCTURAL SET-UP												
No.	connection column to base plate	connection roof to edge beam	connection roof to columns	columns dimension	columns material	additional columns	dowel	dowel connection	layer A	layer E	shear plate in frame corner	pre-tension of columns
1	fixed connection	only transferring vertical loads	fixed connection	600/100mm	Stora Enso LVL (E-Module 13800 N/mm²)	no	pair of dowels 60/100mm 0.00 mm slippage	transferring only shear and normal forces through dowel connection	lengthwise	no	no	no
2	fixed connection	only transferring vertical loads	fixed connection	600/100mm	Stora Enso LVL (E-Module 13800 N/mm²)	no	pair of dowels 60/100mm 1.00 mm slippage	transferring only shear and normal forces through dowel connection	lengthwise	no	no	no
3	fixed connection	only transferring vertical loads	fixed connection	600/100mm	Stora Enso LVL (E-Module 13800 N/mm²)	no	pair of dowels 60/100mm 0.00 mm slippage	transferring only shear and normal forces through dowel connection	crosswise	no	no	no
4	fixed connection	only transferring vertical loads	fixed connection	600/100mm	Stora Enso LVL (E-Module 13800 N/mm²)	no	pair of dowels 60/100mm 0.00 mm slippage	transferring only shear and normal forces through dowel connection	crosswise	yes 5 x 30 mm LVL	no	no
5	fixed connection	only transferring vertical loads	fixed connection	600/100mm	Stora Enso LVL (E-Module 13800 N/mm²)	no	pair of dowels 60/100mm 0.00 mm slippage	transferring all possible forces through dowel connection	crosswise	yes 5 x 30 mm LVL	no	no
6	fixed connection	only transferring vertical loads	fixed connection	600/100mm (along long walls)	Stora Enso LVL (E-Module 13800 N/mm²)	no	pair of dowels 60/100mm 0.00 mm slippage	transferring only shear and normal forces through dowel connection	crosswise	yes 5 x 30 mm LVL	no	no
7	fixed connection	only transferring vertical loads	fixed connection	600/100mm	Stora Enso LVL (E-Module 13800 N/mm²)	no	pair of dowels 60/100mm 0.00 mm slippage	transferring only shear and normal forces through dowel connection	crosswise	yes 5 x 30 mm LVL	yes	no
11	fixed connection	only transferring vertical loads	fixed connection	600/100mm (along long walls)	Stora Enso LVL (E-Module 13800 N/mm²)	no	pair of dowels 60/100mm 0.00 mm slippage	transferring only shear and normal forces through dowel connection	crosswise	yes 5 x 30 mm LVL	yes	no
12	fixed connection	only transferring vertical loads	fixed connection	600/100mm (along long walls)	Stora Enso LVL (E-Module 13800 N/mm²)	no	pair of dowels 60/100mm 0.00 mm slippage	transferring only shear and normal forces through dowel connection	crosswise	yes 5 x 30 mm LVL	yes	no
13	fixed connection	only transferring vertical loads	fixed connection	600/100mm (along long walls)	Stora Enso LVL (E-Module 13800 N/mm²)	no	pair of dowels 60/100mm 0.00 mm slippage	transferring only shear and normal forces through dowel connection	crosswise	yes 5 x 30 mm LVL	yes	no
14	fixed connection	only transferring vertical loads	fixed connection	600/100mm (along long walls)	Stora Enso LVL (E-Module 13800 N/mm²)	yes 2 additional columns next to door to Wälschenberghallen Stora Enso LVL (grid line C x D)	pair of dowels 60/100mm 0.00 mm slippage	transferring only shear and normal forces through dowel connection	crosswise	yes 5 x 30 mm LVL	yes	no
15	fixed connection	only transferring vertical loads	fixed connection	600/100mm (along long walls)	Stora Enso LVL (E-Module 13800 N/mm²)	yes 2 additional columns next to door to Wälschenberghallen Stora Enso LVL (grid line C x D)	pair of dowels 60/100mm 0.00 mm slippage	transferring all possible forces through dowel connection	crosswise	yes 5 x 30 mm LVL	yes	no
16	fixed connection	only transferring vertical loads	fixed connection	600/100mm (along long walls)	Stora Enso LVL (E-Module 13800 N/mm²)	yes 2 additional columns next to door to Wälschenberghallen Stora Enso LVL (grid line C x D)	pair of dowels 60/100mm 0.00 mm slippage	transferring only shear and normal forces through dowel connection	crosswise	yes 5 x 30 mm LVL	yes	no
17	fixed connection	only transferring vertical loads	fixed connection	600/1200mm (along long walls)	Stora Enso LVL (E-Module 13800 N/mm²)	yes 2 additional columns next to door to Wälschenberghallen Stora Enso LVL (grid line C x D)	pair of dowels 60/100mm 0.00 mm slippage	transferring only shear and normal forces through dowel connection	crosswise	yes 5 x 30 mm LVL	yes	no
21	fixed connection	only transferring vertical loads	fixed connection	600/100mm (along long walls)	Stora Enso LVL (E-Module 13800 N/mm²)	yes 2 additional columns next to door to Wälschenberghallen Pullmauer Baubuche (grid line C x D)	pair of dowels 60/100mm 0.00 mm slippage	transferring only shear and normal forces through dowel connection	crosswise	yes 5 x 30 mm LVL	no	no
22	fixed connection	only transferring vertical loads	fixed connection	600/100mm (along long walls)	Pullmauer Baubuche (E-Module 13800 N/mm²)	no	pair of dowels 60/100mm 0.00 mm slippage	transferring only shear and normal forces through dowel connection	crosswise	yes 5 x 30 mm LVL	no	no

Limits specified in the Basis Document			
L/100	L/150	H/75	H/75
2000/150	2000/150	3000/75	3000/75
167 mm	167 mm	93 mm	93 mm

RESULTS											
STRESS RESULTANTS				DEFORMATION							
dowel max. shear force longshore (force per dowel pair)	max. bending moment at bottom of column	max. deformation vertical high pitched area (grid line C)	max. deformation vertical low pitched area (grid line G)	max. deformation horizontal on top of edge beam Wälschenberghallen (grid line G)	max. deformation horizontal on top of edge beam long facade (grid line G)						
ULS (design values)				SLS (S.C. 1.00*(g+a)+1.00*p+1.00*v+0.70*(e+0.02)*w) incl. Creeping							
627 kN	100 %	1500 kNm	100 %	91 mm	100 %	240 mm	100 %	210 mm	100 %	49 mm	100 %
136 kN	88 %	2100 kNm	111 %	103 mm	113 %	275 mm	115 %	236 mm	112 %	57 mm	114 %
592 kN	98 %	1800 kNm	95 %	101 mm	111 %	237 mm	99 %	213 mm	101 %	46 mm	94 %
435 kN	72 %	1150 kNm	71 %	80 mm	88 %	152 mm	83 %	141 mm	87 %	28 mm	87 %
397 kN	65 %	1280 kNm	67 %	76 mm	84 %	143 mm	80 %	132 mm	81 %	26 mm	83 %
380 kN	63 %	1780 kNm	94 %	65 mm	71 %	122 mm	51 %	101 mm	48 %	27 mm	55 %
425 kN	70 %	1300 kNm	68 %	78 mm	86 %	149 mm	62 %	133 mm	83 %	28 mm	87 %
371 kN	61 %	1710 kNm	90 %	63 mm	69 %	119 mm	50 %	96 mm	46 %	27 mm	55 %
327 kN	56 %	2130 kNm	112 %	54 mm	59 %	104 mm	49 %	75 mm	36 %	25 mm	51 %
310 kN	51 %	2550 kNm	134 %	46 mm	51 %	92 mm	38 %	59 mm	28 %	24 mm	49 %
361 kN	59 %	1860 kNm	98 %	55 mm	60 %	119 mm	50 %	86 mm	41 %	27 mm	55 %
330 kN	54 %	1780 kNm	94 %	51 mm	56 %	110 mm	46 %	82 mm	39 %	25 mm	51 %
327 kN	54 %	2400 kNm	126 %	45 mm	49 %	104 mm	49 %	66 mm	31 %	25 mm	51 %
300 kN	49 %	2610 kNm	148 %	38 mm	42 %	92 mm	38 %	54 mm	26 %	24 mm	49 %
370 kN	61 %	1980 kNm	104 %	56 mm	62 %	122 mm	51 %	92 mm	44 %	27 mm	55 %
362 kN	60 %	1900 kNm	100 %	57 mm	63 %	112 mm	47 %	90 mm	43 %	25 mm	51 %

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Produktion



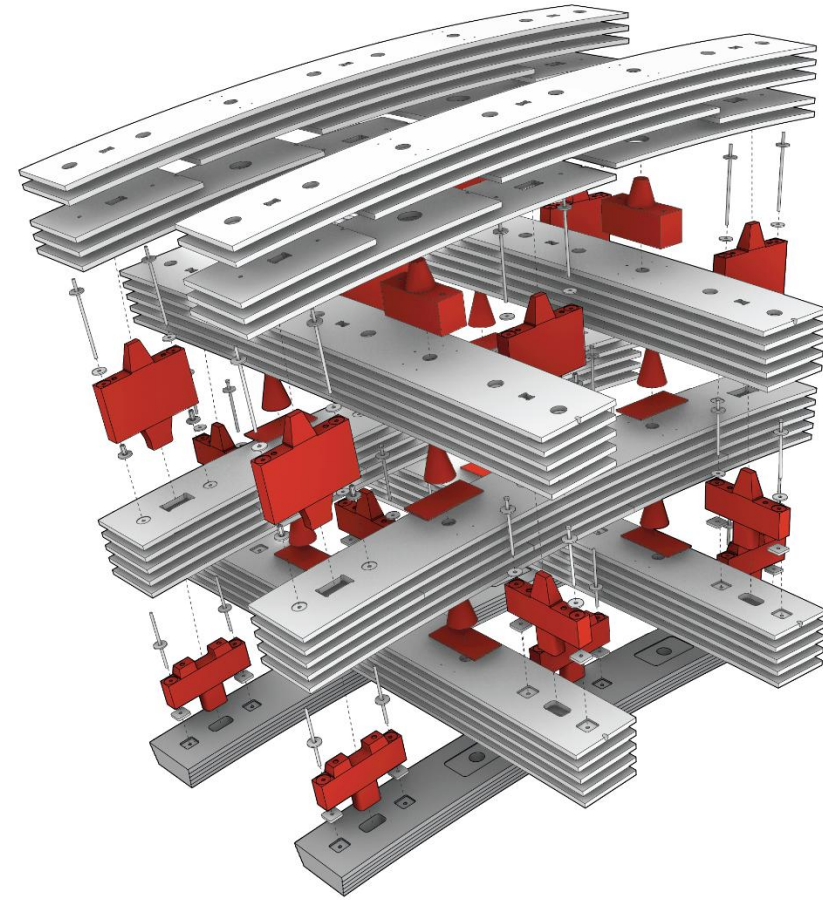
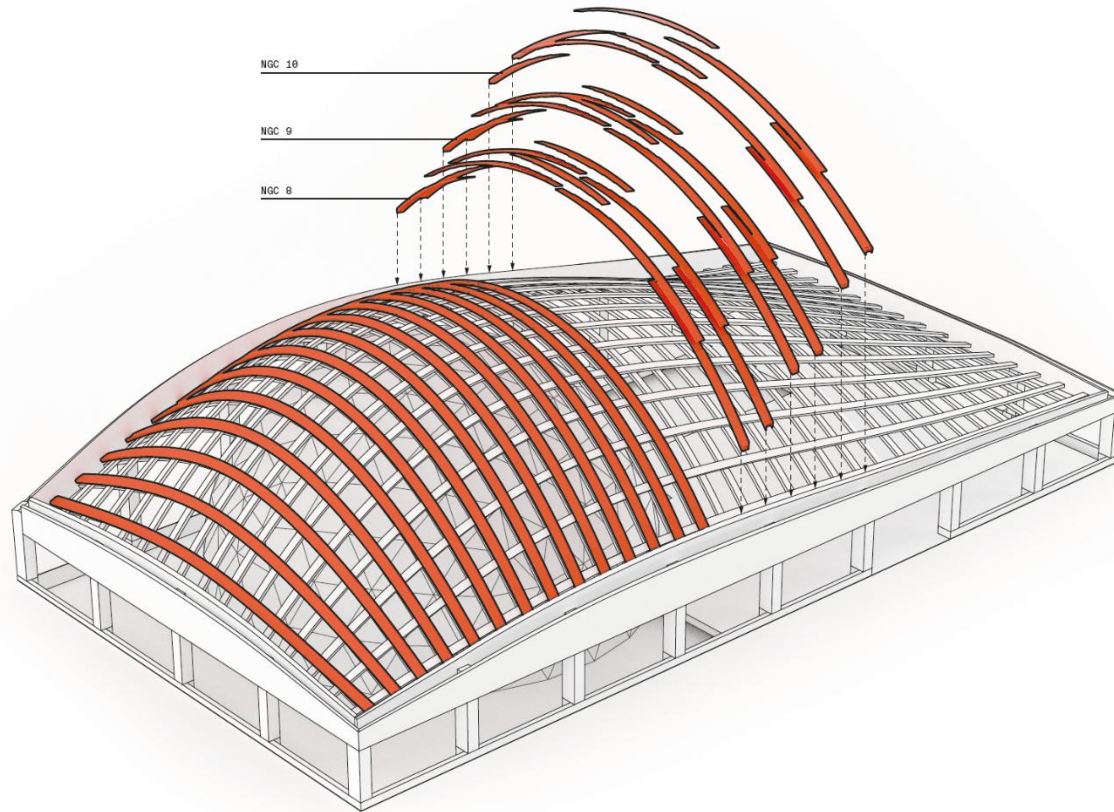
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Mockup



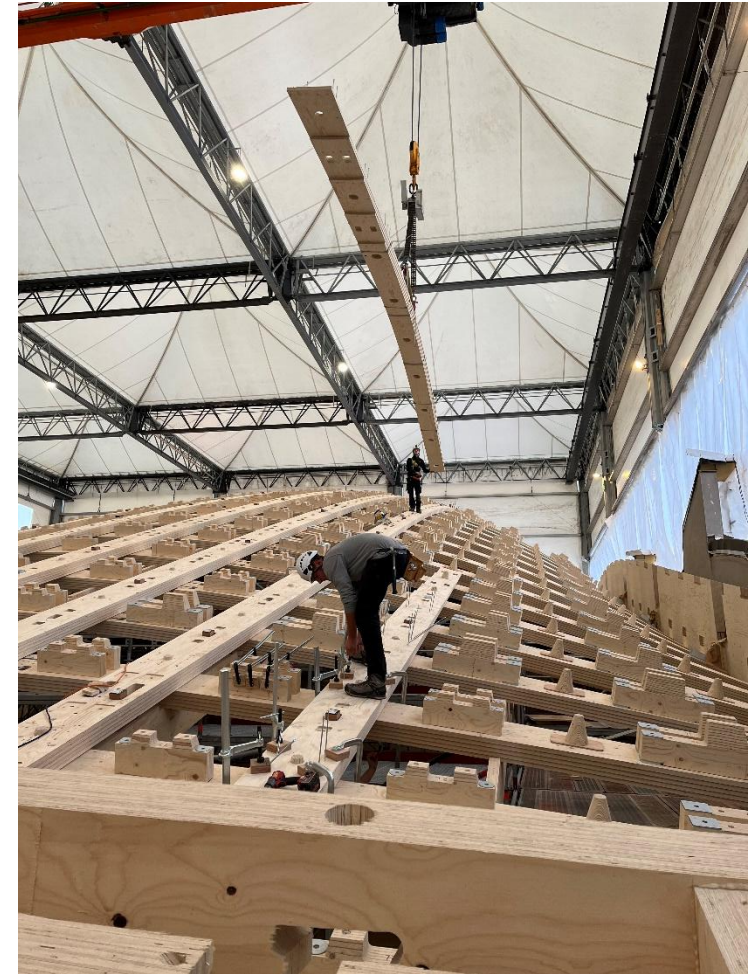
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Montage



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Montage











Stefan Rick
vertreten durch Philipp Schmon