

Load-bearing behaviour of partially threaded screws in hardwood

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Introduction

Karlsruhe Institute of Technology

- Today: complex buildings
 - Taller constructions, larger spans, higher loads
- Solutions for MORE efficient buildings
 - Hardwood: high potential construction material
- Project hardwood_joint
 - Innovative solutions for connection design
 - Knowledge gap: hardwood → softwood?
 - → Use full potential of hardwood members
 - → Obtain high-performance connections



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Challenges

- Connection behaviour
 - Failure modes
- Appropriate design rules
 - Load-bearing capacity
 - Geometric conditions

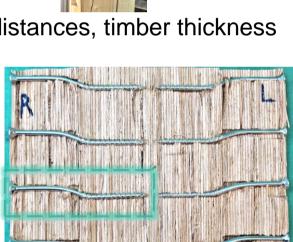
- ductility criterion
- $F_{V.R}$
- spacing, edge distances, timber thickness



- Single screws (Multi-stage tests)
 - Deformation process of screws
 - Initial adhesion
- Groups of screws







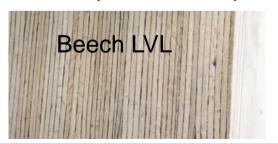
Materials



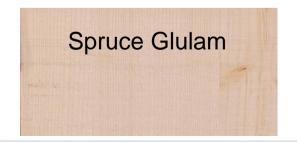
- Partially threaded self-tapping screws:
 - Predrilled insertion 8x160/80mm, countersunk head
 - Non predrilled insertion 8x160/100mm, washer head



Wood species and products



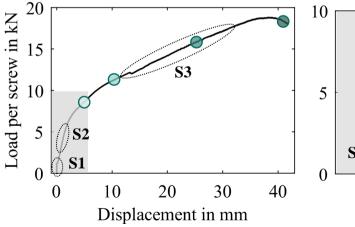


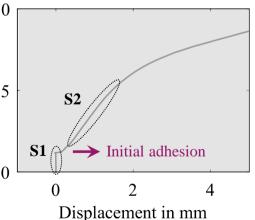


Multi-stage tests



- Single-shear connection tests
- Test termination after reaching displacement stages
 - 5, 10, 25 mm and failure





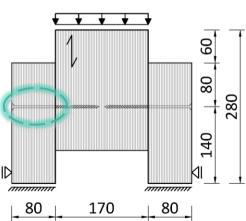
- Initial adhesion = Load at detaching point
- Stiffness and connection behavior: section S1 – S3

Test setup

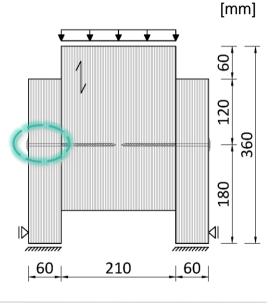




predrilled

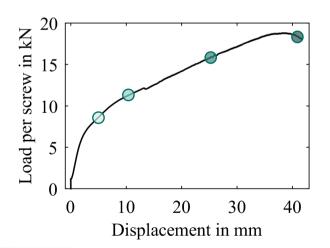


non predrilled



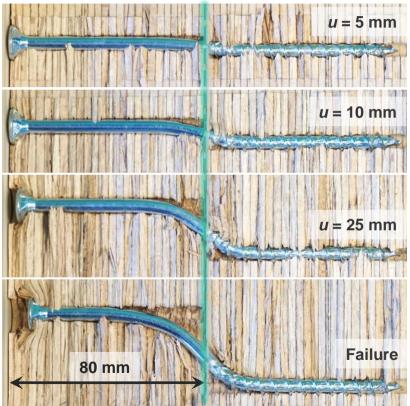
Results in Beech LVL

- Incremental deformation process
 - Plastic hinges starting at u = 10mm
 - Larger displacements: head pull-in
 - Difference: shank and thread







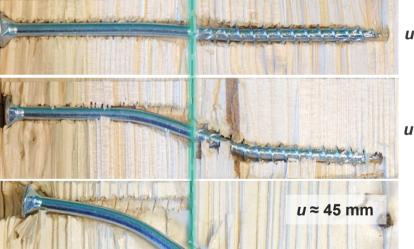




softwood

hardwood

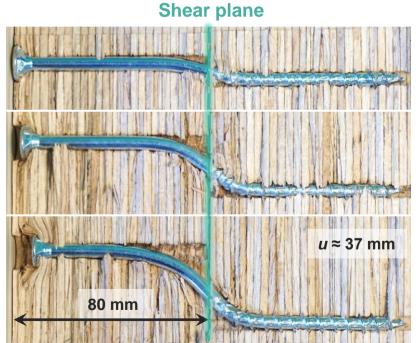
Shear plane



u = 10 mm



Failure



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KIT - Timber Structures and Building Constructions

80 mm

Initial adhesion



■ Prestress of screws → adhesive forces in shear plane



Manual modification of screw insertion

screw 90°/180° overtightened

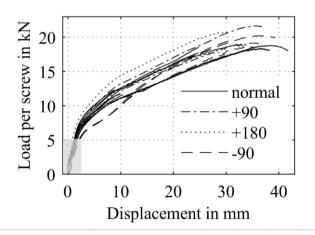
+90° / +180° →

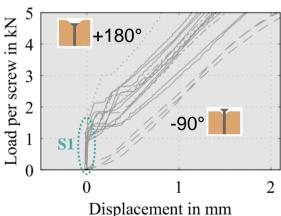


screw 90°/180° turned back

-90° / -180° →







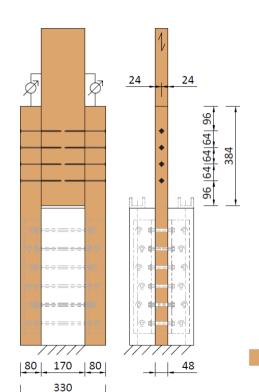
Results of predrilled screws

Groups of screws

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- Test setup
 - Beech LVL & Birch Glulam
 - Single shear connections
 - Geometry

	distance		in mm
predrilled	a_1	8 <i>d</i>	64
	a_2	3 <i>d</i>	24
	a _{3.t}	12 <i>d</i>	96
pr	a _{4,c}	3 <i>d</i>	24
þ	a_1	15 <i>d</i>	120
non predrilled	a_2	7 <i>d</i>	56
n ed	$a_{3,\mathrm{t}}^{-}$	20 <i>d</i>	160
pr	a _{4.c}	7 <i>d</i>	56







One row

Two rows

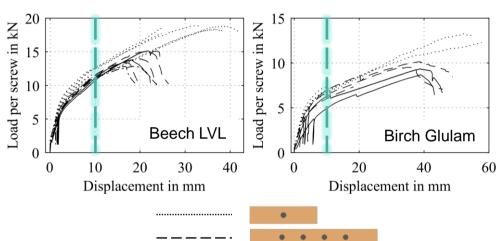


Groups of screws



Results for predrilled screws

Load per screw	Beech LVL	Birch Glulam
and shear plane	Load in kN at $u = 10 \text{ mm}$	
multi-stage tests	12.0	7.02
• • • •	11.4	6.59
• • • •	10.9	5.63
F _{V,R}	9.69	5.74



- F_{V,R} calculated with input parameters
 - Tests: f_h , M_y , f_{ax} , f_{head}
 - Rope effect min (100% F_V ; 25% F_{ax})

Failure modes











- Splitting in grain direction
- Tension perp. to grain in middle member, followed by block-shear failure (birch glulam)





Screw failure in shear plane or plastic hinge

Conclusion



- In general: Current design rules are appropriate for hardwood
- Durability of initial adhesion is debatable
- Non-predrilled screw insertion in hardwood is difficult
 - → Challenge on site
 - → Predrilling is recommended
- Prevent failure perp. to grain of the middle member
- Ductile behaviour: u = 10 mm for screw connections should be achieved
 - → Adjustment of spacing a₁
 - → Guarantee plastic hinges



Thank you for your attention!

ForestValue



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